

- c. Sample results less than the laboratory's MDL shall be reported as "Not Detected," or ND.
 - d. Dischargers are to instruct laboratories to establish calibration standards so that the ML value (or its equivalent if there is differential treatment of samples relative to calibration standards) is the lowest calibration standard. At no time is the Discharger to use analytical data derived from *extrapolation* beyond the lowest point of the calibration curve.
5. The Discharger shall submit SMRs in accordance with the following requirements:
- a. The Discharger shall arrange all reported data in a tabular format. The data shall be summarized to clearly illustrate whether the facility is operating in compliance with interim and/or final effluent limitations. The Discharger is not required to duplicate the submittal of data that is entered in a tabular format within CIWQS. When electronic submittal of data is required and CIWQS does not provide for entry into a tabular format within the system, the Discharger shall electronically submit the data in a tabular format as an attachment.
 - b. The Discharger shall attach a cover letter to the SMR. The information contained in the cover letter shall clearly identify violations of the WDRs; discuss corrective actions taken or planned; and the proposed time schedule for corrective actions. Identified violations must include a description of the requirement that was violated and a description of the violation.
 - c. SMRs must be submitted to the Regional Water Board, signed and certified as required by the Standard Provisions (Attachment D), to the address listed below:

C. NOTIFICATION

1. The Discharger shall notify the Executive Officer in writing prior to discharge of any chemical that may be toxic to aquatic life. Such notification shall include:
 - a. Name and general composition of the chemical,
 - b. Frequency of use,
 - c. Quantities to be used,
 - d. Proposed discharge concentrations, and
 - e. EPA registration number, if applicable.

No discharge of such chemical shall be made prior to obtaining the Executive Officer's approval.

2. The Discharger shall notify the Regional Board via telephone and/or fax within 24 hours of noticing an exceedance above the effluent limits in Order No. R4-2008-0032. The Discharger shall provide to the Regional Board within 14 days of observing the exceedance a detailed statement of the actions undertaken or proposed that will bring the discharge into full compliance with the requirements and submit a timetable for correction.

D. MONITORING FREQUENCIES ADJUSTMENT

Monitoring frequencies may be adjusted by the Executive Officer to a less frequent basis if the Discharger requests same and the request is backed by statistical trends of monitoring data submitted.

E. SELF MONITORING REPORTS (SMRs)

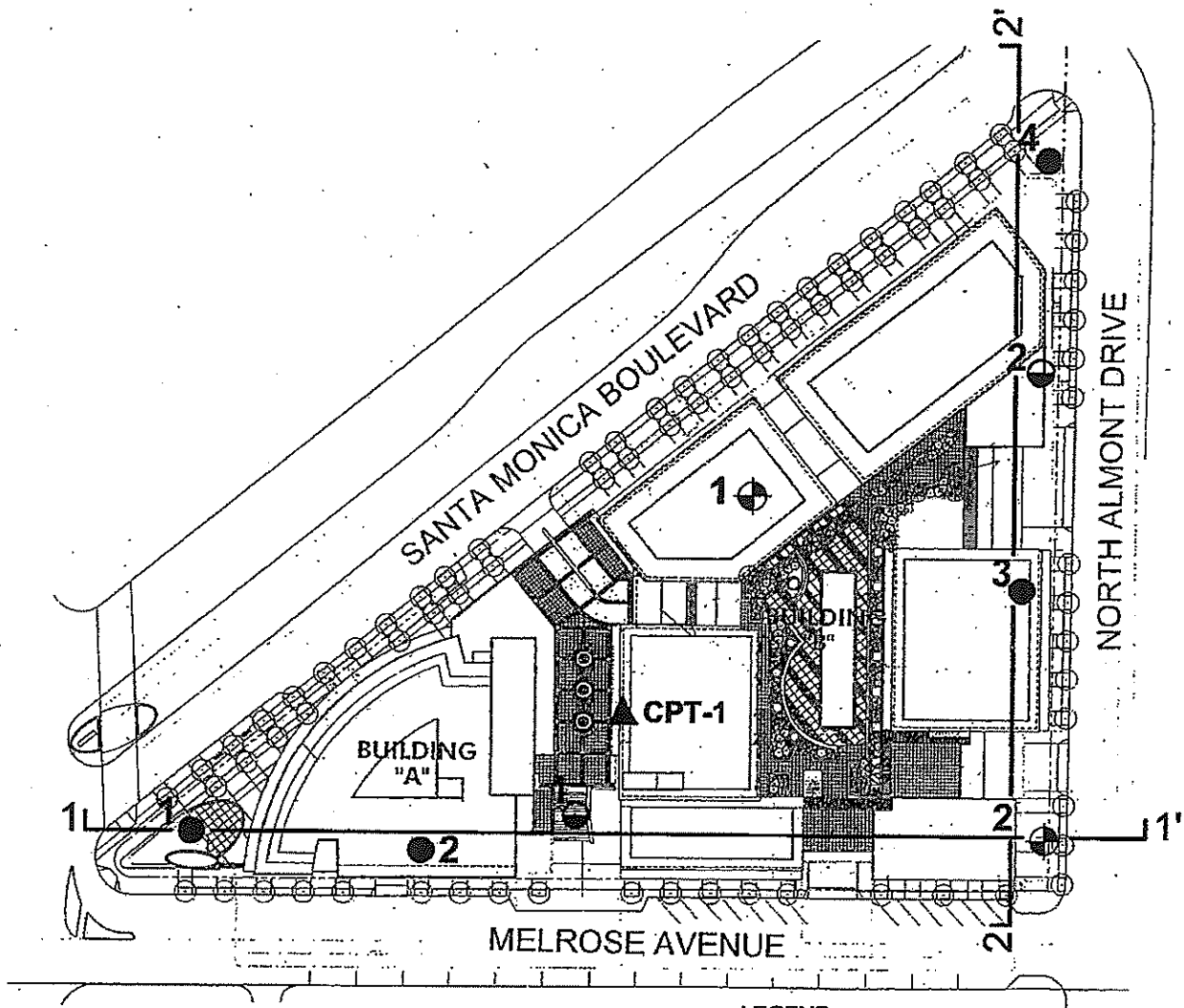
SMRs must be signed and certified as required by the standard provisions (Attachment D). The Discharge shall submit the original SMR to the address listed below:

California Regional Water Quality Control Board
Los Angeles Region
320 W. 4th Street, Suite 200
Los Angeles, CA 90013
Attention: Information and Technology Unit.

APPENDIX B

BORING AND CPT LOGS

WELL SURVEY DATA



LEGEND:

- 2 CURRENT PROJECT 4953-08-0811
- 4 PRIOR PROJECT 4953-06-2101
- 2 PRIOR PROJECT A-85280
- BORING LOCATION AND NUMBER
- CPT-1 CURRENT PROJECT 4953-08-0811
- CONE PENETRATION TEST LOCATION AND NUMBER

2' _____ 2' SECTION LINE

REFERENCE:
PLOT PLAN PROVIDED BY
STUDIO ONELEVEN
ARCHITECTS, UNDATED.



MACTEC

5829 E. SLAUSON AVENUE
LOS ANGELES, CALIFORNIA 90040
(323) 889-5300 FAX (323) 889-5396

PLOT PLAN

PROPOSED MELROSE TRIANGLE MIXED-USE PROJECT
WEST HOLLYWOOD, CALIFORNIA

FIGURE 2

PROJECT NO. 4953-08-0811	REVISION:
DATE: 6/03/08	
SCALE: AS NOTED	
DWG BY: TT	CHECKED BY: <i>UT</i>

BORING 1

DATE DRILLED: October 3, 2006
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 222.5**

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

ELEVATION (ft)	DEPTH (ft)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.	DESCRIPTION
222.5	0					SM	3" Thick Asphalt Concrete
220	2.5					CL	SILTY SAND - slightly moist, brown
215	7.5	14.3	14.3	119	8		SANDY LEAN CLAY - slightly moist, brown
210	12.5	12.8	12.8	110	8		CLAYEY SAND - slightly moist, brown
205	17.5	11.7	11.7	116	9		CLAYEY SAND - slightly moist, brown
200	22.5	6					31.9% Passing No. 200 sieve Becomes more gravelly, few cobbles
195	27.5	13.3	13.3	118	9		CLAYEY SAND - slightly moist, brown
190	32.5	17					31.9% Passing No. 200 sieve Becomes more gravelly, few cobbles
185	37.5	11.7	11.7	116	27	SM	SILTY SAND - moist, brown
180	42.5	25					Becomes sandier, few gravel
175	47.5	19.0	19.0	110	15	CL	SANDY LEAN CLAY - moist, brown, few gravel
170	52.5	16					LL=28, PI=13
165	57.5	20.5	20.5	110	15		SANDY LEAN CLAY - moist, brown, few gravel
160	62.5	12					Becomes less gravelly

B12SOIL CRANDALL/DECIMAL ELEV. 62101.GPI LAW CRAN/GDT 7/15/08

(CONTINUED ON FOLLOWING FIGURE)

Field Tech: CMC
 Prepared By: AO
 Checked By: LT

Melrose Triangle
 West Hollywood, California



LOG OF BORING
 Project: 4953-06-2101 Figure: A-1.1a

BORING 1 (Continued)

DATE DRILLED: October 3, 2006
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 222.5**

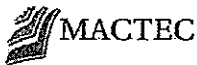
THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

ELEVATION (ft)	DEPTH (ft)	N ^o VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.	DESCRIPTION
180	18		13.2	117	35	SW	WELL-GRADED SAND - moist to very moist, brown, some gravel Becomes more gravelly
145						CL-ML	SILTY CLAY - moist, brown
175			13.4	118	43	SM	SILTY SAND - brown, saturated, few gravel
175	87					SW	WELL-GRADED SAND - wet, brown, some gravel
170							brownish gray
165			13.3	118	50		
160	50					CL	SANDY LEAN CLAY - moist, brown, few fine gravel
160						SM	SILTY SAND - moist, brown, few gravel
155			11.6	124	33	CL	SANDY LEAN CLAY - moist, brown, few gravel
150	45						Layers of sand and gravel
145			13.7	117	43		
145						SC	CLAYEY SAND - moist, brown Becomes gravelly
80	61						

(CONTINUED ON FOLLOWING FIGURE)

Field Tech: CMC
 Prepared By: AO
 Checked By: *UA*

Melrose Triangle
 West Hollywood, California



LOG OF BORING

Project: 4953-06-2101 Figure: A-1.1b

B12SOIL CRANDALL(DECIMAL ELE) 62101.GPJ LAW CRAN.GDT 7/15/08

BORING 1 (Continued)

DATE DRILLED: October 3, 2006
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 222.5**

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

ELEVATION (ft)	DEPTH (ft)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.
140						SC
85			15.1	118	35	
135		83				CL
90						
130						SW
95			12.8	116	50	
125						
100			11.7	117	50	
120						
105						
115						
110						
110						
115						
105						
120						

CLAYEY SAND - moist, brown, few gravel

SANDY LEAN CLAY - moist, brown, some gravel

WELL-GRADED SAND - wet, brown

END BORING AT 100 FEET

NOTES: Mud used during drilling process. Bailed hole to 50 feet +/- clean water. Water level at 31.5 feet at completion. Some caving at 41 feet. Water at 28.5 feet after 15 mins and 25 mins. Stable water possibly under hydrostatic pressure. Some slight seepage at (24' +/-, 21' - 24'). First noticeable water at 39.5 feet. Boring grouted with a cement-bentonite mixture.

CMU Auto-trip hammer used in obtaining the "N-VALUE" Standard Penetration Test (SPT) blowcounts.

* Number of blows required to drive the Crandall sampler 12 inches using a 300 pound hammer falling 24 inches.

**Elevations are approximate and surveyed in prior to drilling.

Field Tech: CMC
 Prepared By: AO
 Checked By: LT

Melrose Triangle
 West Hollywood, California



LOG OF BORING

Project: 4953-06-2101 Figure: A-1.1c

E12SOIL CRANDALL(DECIMAL ELE) 62101.GPI LAW CRAN.GDT 7/15/08

BORING 2

DATE DRILLED: October 3, 2006
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 217.8**

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

ELEVATION (ft)	DEPTH (ft)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.
217.8	0					SM
215	2.8					
210	7.8		13.5	118	14	CL-ML
205	12.8	14	14.4	114	4	CL
200	17.8	6	9.6	114	8	SC
195	22.8				15	SW
190	27.8	36	20.8	106	15	CL-ML
185	32.8	9	17.6	112	8	ML
180	37.8	30	10.8	122	31	SW

3" Thick Asphalt Concrete
 FILL - SILTY SAND - moist, brown, few gravel and cobbles

SILTY CLAY - moist, brown, trace gravel

SANDY LEAN CLAY - moist, dark brown, some gravel

CLAYEY SAND - with fine gravel, moist, brown

WELL-GRADED SAND - wet, light brown, some gravel

No Recovery
 Becomes less gravelly

SILTY CLAY - moist, brown, trace gravel

SANDY SILT - moist, brown, fine to medium grained sand

WELL-GRADED SAND - saturated, brown, some gravel

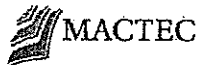
Becomes less gravelly

B12SOIL CRANDALL (DECIMAL ELE) 62101.GRI LAW CRAN.GDT 7/15/08

(CONTINUED ON FOLLOWING FIGURE)

Field Tech: CMC
 Prepared By: AO
 Checked By: U

Melrose Triangle
 West Hollywood, California

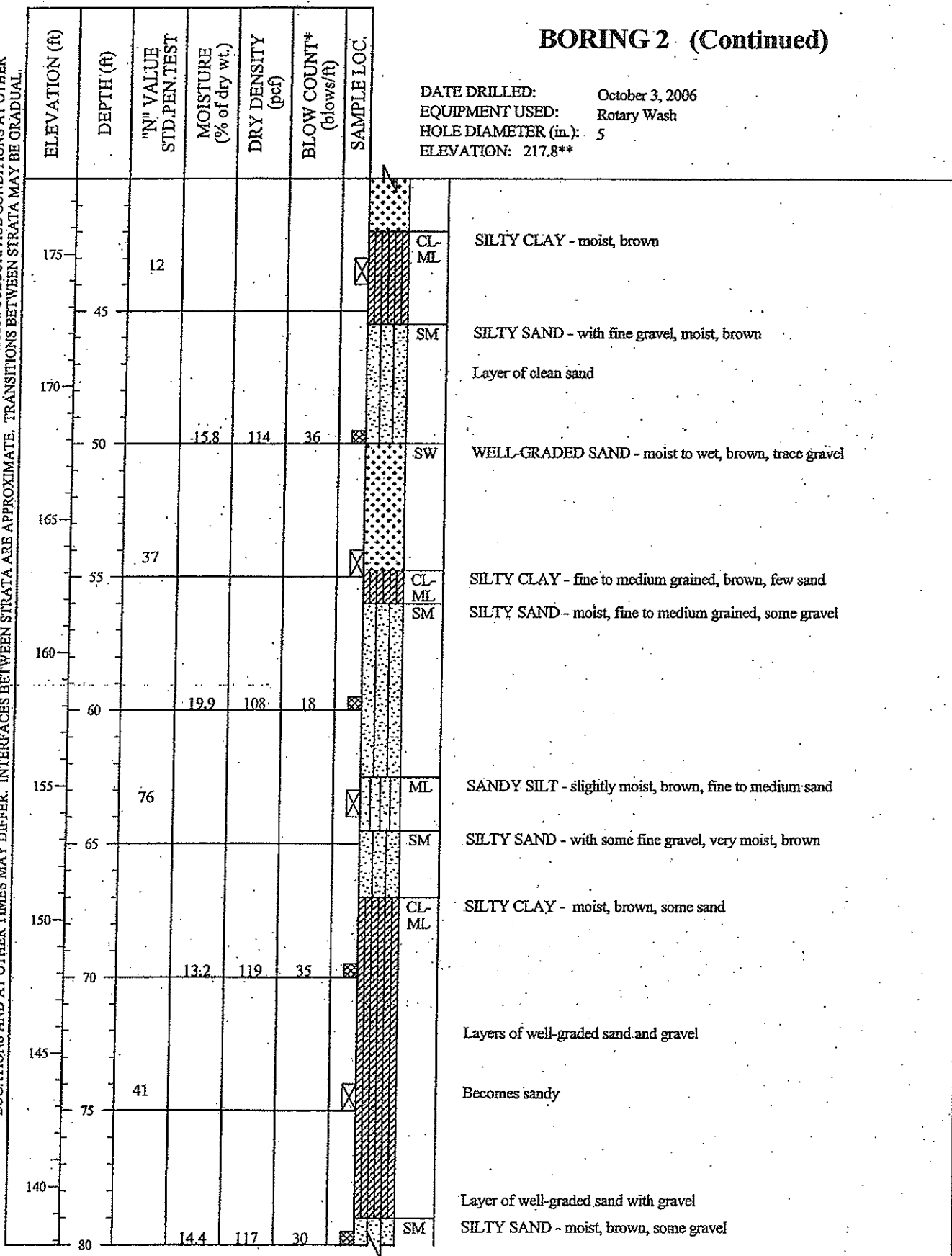


LOG OF BORING
 Project: 4953-06-2101 Figure: A-1.2a

BORING 2 (Continued)

DATE DRILLED: October 3, 2006
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 217.8**

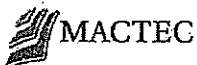
THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.



(CONTINUED ON FOLLOWING FIGURE)

Field Tech: CMC
 Prepared By: AO
 Checked By: *[Signature]*

Melrose Triangle
 West Hollywood, California



LOG OF BORING
 Project: 4953-06-2101 Figure: A-1.2b

B12SOIL CRANDALL(DECIMAL_ELE) 62101.GPJ LAW CRAN.GDT 7/15/08

BORING 2 (Continued)

DATE DRILLED: October 3, 2006
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 217.8**

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

ELEVATION (ft)	DEPTH (ft)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.
135		39				
130						SC
90		9.5	131	50		
125						SW
95		50/5"				
100		11.2	115	60		
110						
115						
105						
100						
120						

Thin layers of sand and gravel

CLAYEY SAND - moist, brown, trace gravel

WELL-GRADED SAND - saturated, brown, some gravel

END BORING AT 100 FEET

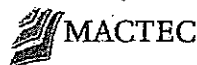
NOTES: Mud used during drilling process. Bailed hole to 35 feet +/- clean water. Water level remaining at 27 feet during bailing. Left hole open overnight. Water at 24 feet after 15 hrs. Some caving at 41 feet. Very coarse gravel at 19 1/2 to 21 1/2 feet, lost circulation. Boring grouted with a cement-bentonite mixture.

CMU Auto-trip hammer used in obtaining the "N-VALUE" Standard Penetration Test (SPT) blowcounts.

Field Tech: CMC
 Prepared By: AO
 Checked By:

B12SOIL CRANDALL(DECIMAL ELE) 0210101 LAW CRANLODT 7/15/08

Melrose Triangle
 West Hollywood, California



LOG OF BORING
 Project: 4953-06-2101 Figure: A-1.2c

BORING 3

DATE DRILLED: October 5, 2006
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 222.1**

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE. REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

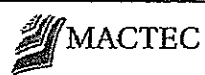
ELEVATION (ft)	DEPTH (ft)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.	DESCRIPTION
222.1	0						4" Thick Asphalt Concrete, 4" Thick Base Course, 4" thick Concrete slab
220	2					CL	SANDY LEAN CLAY - moist, fine to coarse grained, dark brown
215	7	18.3	103	11			
210	12	16.3	112	15			
210	12		11.3	107	8	SM	SILTY SAND - moist, brown
210	12					CL	SANDY LEAN CLAY - moist, brown, fine to medium sand
205	17	13				SW-SM	WELL-GRADED SAND with SILT - moist, brown
205	17	9.8	110	8			
200	22	8				ML	SANDY SILT - very moist, dark brown
200	22	13.3	118	15		SM	SILTY SAND - moist, brown, some gravel
195	27	6				CL-ML	SILTY CLAY - saturated to moist, brown, some sand
190	32	20	19.6	113	16		Becomes stiffer, less saturated
190	32					SW	WELL-GRADED SAND - saturated, brown, some gravel
185	37	15.6	113	23			Disturbed sample Some clay in bit
185	37	7				CL	SANDY LEAN CLAY - very moist, brown, few fine gravel LL=30, PI=14
180	42						

B12SOIL CRANDALL(DECIMAL ELE) 62101.GPI LAW CRAN.GDT 7/15/08

Field Tech: AR
 Prepared By: AO
 Checked By: U

(CONTINUED ON FOLLOWING FIGURE)

Melrose Triangle
 West Hollywood, California



LOG OF BORING
 Project: 4953-06-2101 Figure: A-1.3a

BORING 3 (Continued)

DATE DRILLED: October 5, 2006
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 222.1**

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

ELEVATION (ft)	DEPTH (ft)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.
180	9		17.2	112	15	
175			21.1	106	23	SP
170						
165						
160	19					CL-ML
155					13	
150						
145	54					SW
140						CL
135						SW
130						SC
125			9.6	129	39	
120						
115	41					
110						
105						
100						
95						
90						
85						
80	35		13.6	122	38	CL

POORLY GRADED SAND - very moist, brown
 Siltier with some clay

SILTY CLAY - saturated, brown, some sand and gravel

WELL-GRADED SAND - moist, brown, fine to coarse, some gravel

SANDY LEAN CLAY - moist, brown

WELL-GRADED SAND - brown, some clay

CLAYEY SAND - brown to dark brown, moist, some gravel

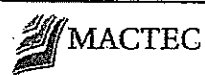
SANDY LEAN CLAY - very moist, black

B12501L CRANDALL (DECIMAL ELE) 62101.GPI LAW CRAN.GDT 7/15/08

(CONTINUED ON FOLLOWING FIGURE)

Field Tech: AR
 Prepared By: AO
 Checked By: *LA*

Melrose Triangle
 West Hollywood, California



LOG OF BORING
 Project: 4953-06-2101 Figure: A-1.3b

BORING 3 (Continued)

DATE DRILLED: October 5, 2006
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 222.1**

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

ELEVATION (ft)	DEPTH (ft)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.
140						
85		13.9	117	48		SC
135		28				
90						
130						
95						
125						
100		11.8	124	65		SM
120						
105						
115						
110						
110						
115						
105						
120						

LL=31, PI=15

CLAYEY SAND - very moist, brown, some gravel

43% Passing No. 200 sieve

SILTY SAND - moist, brown, some gravel

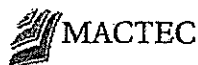
END BORING AT 100 FEET

NOTES: Mud used during drilling process. Well within 5 feet, water at 26.6 feet. Boring grouted with a cement-bentonite mixture.

CMU Auto-trip hammer used in obtaining the "N-VALUE" Standard Penetration Test (SPT) blowcounts.

Field Tech: AR
 Prepared By: AO
 Checked By: *AR*

Melrose Triangle
 West Hollywood, California



LOG OF BORING

Project: 4953-06-2101

Figure: A-1.3c

BORING 4

DATE DRILLED: October 3, 2006
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 213.7**

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

ELEVATION (ft)	DEPTH (ft)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.
213.7	0					CL
210	5		13.8	116	9	SM
205	10	8	11.7	109	5	ML
200	15	5	16.1	109	6	CL
195	20		16.5	113	5	ML
190	25	5				CL
185	30	32	17.7	111	16	SW
180	35	64	12.2	120	15	ML
175	40					

5" Thick Concrete Slab
 SANDY LEAN CLAY - slightly moist, dark brown
 few gravel

SILTY SAND - moist, brown, few gravel

SANDY LEAN CLAY - moist, brown, few gravel
 moderately soft

SANDY SILT - very moist, brown
 Possible seepage at 21.5 to 25 feet
 56.5% Passing No. 200 sieve

SANDY LEAN CLAY - brown, moist

WELL-GRADED SAND - with some gravel, moist, brown
 from 30 feet to 34 feet - siltier layers

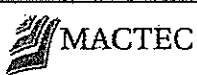
SANDY SILT - very moist to wet, brown, firm, some gravel

BIS/JOIL CRANDALL(DECIMAL, E) 62101.GPI LAW CRAN/GDT 7/15/08

(CONTINUED ON FOLLOWING FIGURE)

Field Tech: CMC
 Prepared By: AO
 Checked By: *U*

Melrose Triangle
 West Hollywood, California



LOG OF BORING

Project: 4953-06-2101 Figure: A-1.4a

BORING 4 (Continued)

DATE DRILLED: October 3, 2006
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 213.7**

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

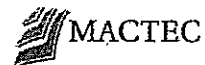
ELEVATION (ft)	DEPTH (ft)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.	DESCRIPTION
170	45	50/5"	11.1	117	45	SW	WELL-GRADED SAND - very moist, brown more gravel and cobbles large cobbles
165	50	65	14.6	116	50	CL ML SW	SILTY CLAY - saturated, brown WELL-GRADED SAND - moist, brown, some gravel
160	55						Some larger gravel and small cobbles
155	60	26				CL	SANDY LEAN CLAY - moist, brown, some gravel
150	65		14.6	115	40		
145	70	50				SC	CLAYEY SAND - slightly moist, brown
140	75		15.2	116	35	CL	SANDY LEAN CLAY - moist, black
135	80	31					

(CONTINUED ON FOLLOWING FIGURE)

Field Tech: CMC
 Prepared By: AO
 Checked By: U

B12SOIL CRANDALL(DECIMAL ELE) 62101.GPJ LAW CRAN.GDT 7/15/08

Melrose Triangle
 West Hollywood, California



LOG OF BORING

Project: 4953-06-2101 Figure: A-1.4b

BORING 4 (Continued)

DATE DRILLED: October 3, 2006
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 213.7**

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

ELEVATION (ft)	DEPTH (ft)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.
130						CL-ML
85			25.9	101	21	
125						CL
90		55				
120						
95			12.7	124	50	
115						SM
100		50/4"				
110						
105			15.2	114	50	
105						
110						
100			12.5	124	50	ML
115						
95			9.5	127	50	SW
120						

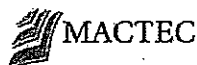
LL=36, PI=19
 SILTY CLAY - gray, moist, brown
 70.3% Passing No. 200 sieve
 SANDY LEAN CLAY - moist, brown, some gravel
 SILTY SAND - slightly moist, brown, few gravel
 SANDY SILT - brown, moist, some gravel
 few small layers of clay
 WELL-GRADED SAND - moist, brown, some gravel

(CONTINUED ON FOLLOWING FIGURE)

Field Tech: CMC
 Prepared By: AO
 Checked By: [Signature]

B12SOIL CRANDALL(DECIMAL ELE) 62101.GPI LAW_CRAN.GDT 7/15/08

Melrose Triangle
 West Hollywood, California



LOG OF BORING

Project: 4953-06-2101

Figure: A-1.4c

BORING 4 (Continued)

DATE DRILLED: October 3, 2006
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 213.7**

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

ELEVATION (ft)	DEPTH (ft)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.
90						
125		192	108	40		
85						
130						
80						
135						
75						
140						
70						
145						
65						
150						
60						
155						
55						
160						

CL SANDY LEAN CLAY - slightly moist, brown

END OF BORING AT 125 FEET

Mud used during drilling process. Bailed hole to 46 feet. Water level remaining at 21.8 feet during bailing. Seepage at 28 feet. Some caving to 48 feet. Boring grouted with a cement-bentonite mixture.

CMU Auto-trip hammer used in obtaining the "N-VALUE" Standard Penetration Test (SPT) blowcounts.

Field Tech: CMC
 Prepared By: AO
 Checked By:

Melrose Triangle
 West Hollywood, California



LOG OF BORING
 Project: 4953-06-2101 Figure: A-1.4d

B125SOIL CRANDALL (DECIMAL ELEV) 62101.GPI LAW CRAN.GDT 7/15/08

BORING 5

DATE DRILLED: May 19-20, 2008
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 223.7**

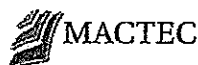
THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE. REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

ELEVATION (ft)	DEPTH (ft)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.	DESCRIPTION
223.7	0					ML	3" Thick Asphalt Concrete
220	5	17.6	106	11		CL	FILL - SILT and CLAY - moist, brown, some sand and gravel
215	10	17.1	114	21		CL	FILL - SANDY CLAY - moist, brown, some gravel
210	15	6				SM	LEAN CLAY - moist, light brown, fine to medium sand
205	20	21.4	99	12		CL	LL = 44, PI = 23
200	25	20				CL	SILTY SAND - moist, light brown, fine to medium
195	30	11.5	111	29		ML	37.8% Passing No. 200 Sieve
190	35	12				CL-ML	LEAN CLAY - moist, light brown, fine to coarse sand
185	40	11.4	124	48		SM	58.9% Passing No. 200 Sieve, LL = 40, PI = 17
						SM	54.3% Passing No. 200 Sieve
						ML	SANDY SILT - moist, light brown, fine to coarse sand
						CL-ML	SILTY CLAY - moist, light brown
						SM	wet, LL = 34, PI = 16
						SM	SILTY SAND - wet, light brown, fine to coarse, some gravel
						SM	24.6% Passing No. 200 Sieve

(CONTINUED ON FOLLOWING FIGURE)

Field Tech: GMC
 Prepared By: LT
 Checked By: JF

Melrose Triangle
 West Hollywood, California



LOG OF BORING

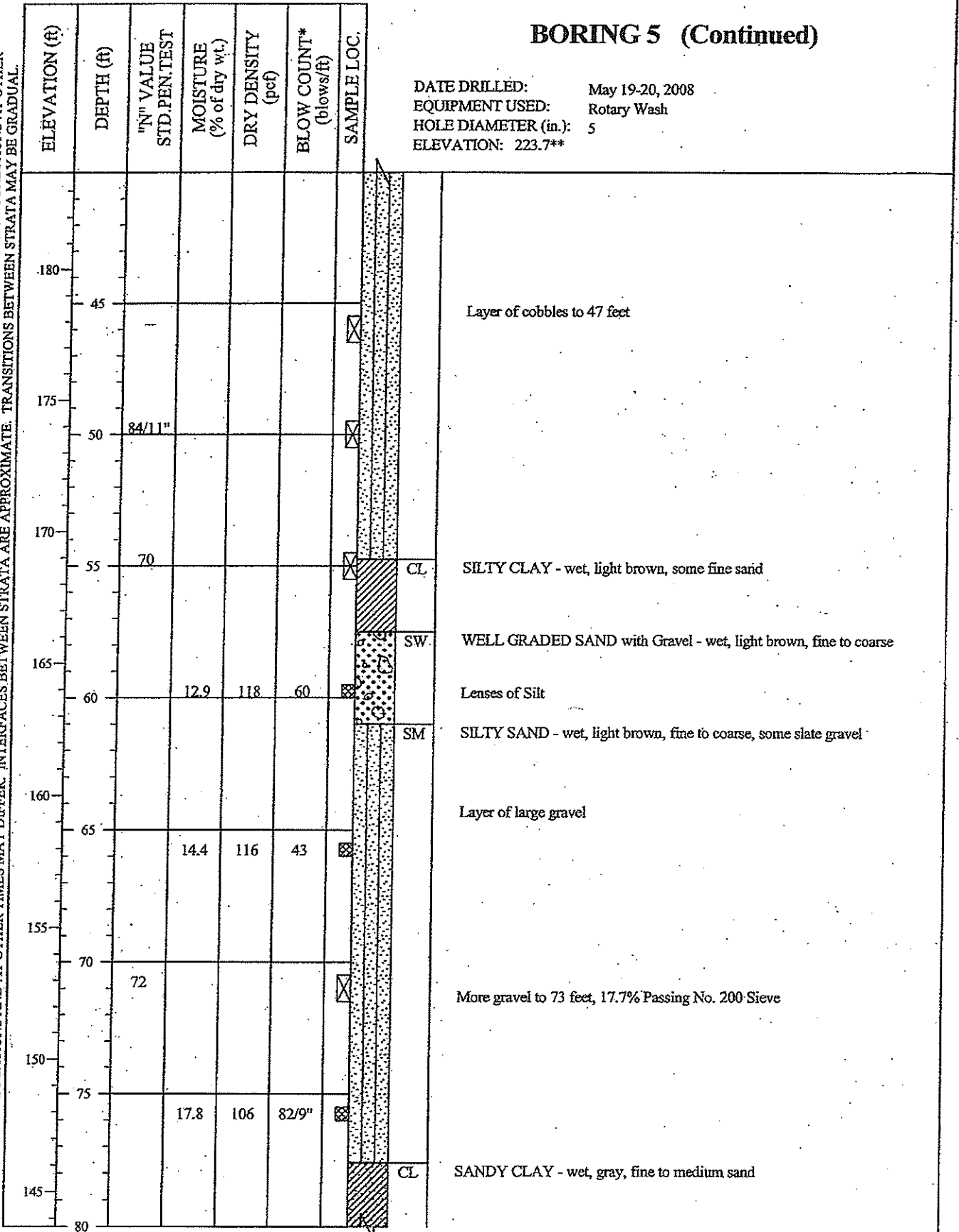
Project: 4953-08-0811 Figure: A-1.5a

BLSOIL CRANDALL(DECIMAL ELE) 80811(OP) LAW CRAN/GDT 7/15/08

BORING 5 (Continued)

DATE DRILLED: May 19-20, 2008
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 223.7**

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.



(CONTINUED ON FOLLOWING FIGURE)

Field Tech: GMC
 Prepared By: LT
 Checked By: JT

Melrose Triangle
 West Hollywood, California



LOG OF BORING

Project: 4953-08-0811

Figure: A-1.5b

BORING 5 (Continued)

DATE DRILLED: May 19-20, 2008
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 223.7**

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

ELEVATION (ft)	DEPTH (ft)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.
140		32				☒
85			20.7	105	37	☒
135						
90						
130						
95		43				☒
125						
100						
120						
105						
115						
110						
115						
105						
120						

END BORING AT 96.5 FEET

NOTES: Mud used during drilling process. Bailed hole to 36 feet. Water level at 33 feet at completion of drilling. Some caving occur in boring. Boring grouted with a cement-bentonite mixture.

CMU Auto-trip hammer used in obtaining the "N-VALUE" Standard Penetration Test (SPT) blowcounts.

* Number of blows required to drive the Crandall sampler 12 inches using a 300 pound hammer falling 24 inches.

**Elevations are approximate and surveyed based on referenced datum.

B12SOIL CRANDALL(DECIMAL ELE) 80811.GPJ LAW CRAN.GDT 7/15/08

Melrose Triangle
West Hollywood, California



LOG OF BORING

Project: 4953-08-0811

Figure: A-1.5c

Field Tech: GMC
 Prepared By: LT
 Checked By: Jf

BORING 6

DATE DRILLED: May 21, 2008
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 209.8**

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

ELEVATION (ft)	DEPTH (ft)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.
209.8	0					ML
205	5		17.2	-	9	SM
200	10	7				CL
195	15				16	SM
190	20	11				SM
185	25				31	ML
180	30	20				ML
175	35				32	SW
170	40					CL

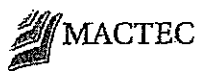
4" Concrete Slab
 FILL - CLAYEY SILT - moist, dark brown, fine to medium sand, some slate fragment
 CLAYEY SILT - moist, brown, fine sand
 SILTY SAND - moist, brown, fine to coarse, some clay
 LEAN CLAY - moist, brown, fine to medium sand
 Thin layer of silty sand, fine to coarse, some rootlets
 SILTY SAND - moist, brown and some dark gray, fine to coarse, some clay, some small slate fragments
 CLAYEY SILT - moist, brown and dark gray, fine to coarse sand
 WELL GRADED SAND - moist, brown, fine to coarse, some gravel
 SANDY LEAN CLAY - moist, brown, fine to coarse

B12501L CRANDALL/DECIMAL ELE/ 80811/GPI LAW CRAN/GDT 7/15/08

(CONTINUED ON FOLLOWING FIGURE)

Field Tech: AR
 Prepared By: LT
 Checked By: JT

Melrose Triangle
 West Hollywood, California



LOG OF BORING
 Project: 4953-08-0811 Figure: A-1.6a

BORING 6 (Continued)

DATE DRILLED: May 21, 2008
 EQUIPMENT USED: Rotary Wash
 HOLE DIAMETER (in.): 5
 ELEVATION: 209.8**

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. LATITUDE AND LONGITUDE OF BORING LOCATION SHOWN ON LOGS ARE APPROXIMATE; REFER TO PLOT PLAN FOR MORE ACCURATE LOCATION INFORMATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BETWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

ELEVATION (ft)	DEPTH (ft)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (pcf)	BLOW COUNT* (blows/ft)	SAMPLE LOC.
165	45	17				SM
160	50	36				
155	55	43				
150	60	42				
145	65	51				
140	70					
135	75					
130	80					

LL = 32, PI = 12

SILTY SAND - moist, brownish gray, very fine to medium

16.6% Passing No. 200 Sieve

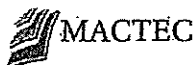
More gravel

END BORING AT 73 FEET DUE TO DIFFICULT DRILLING

NOTES: Mud used during drilling process. Bailed hole to 22 feet. Ground-water measured at 21 feet after completion of drilling. Boring grouted with a cement-bentonite mixture.

Field Tech: AR
 Prepared By: LT
 Checked By: JF

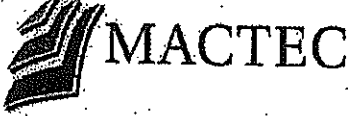
Melrose Triangle
 West Hollywood, California



LOG OF BORING

Project 4953-08-0811

Figure: A-1.6b

MAJOR DIVISIONS			GROUP SYMBOLS	TYPICAL NAMES	Undisturbed Sample	Auger Cuttings																																		
COARSE GRAINED SOILS (More than 50% of material is LARGER than No. 200 sieve size)	GRAVELS (More than 50% of coarse fraction is LARGER than the No. 4 sieve size)	CLEAN GRAVELS (Little or no fines)	GW	Well graded gravels, gravel - sand mixtures, little or no fines.	Standard Penetration Test	Auger Cuttings																																		
		GRAVELS WITH FINES (Appreciable amount of fines)	GP	Poorly graded gravels or grave - sand mixtures, little or no fines.		Rock Core	Bulk Sample																																	
			GM	Silty gravels, gravel - sand - silt mixtures.	Dilatometer	Crandall Sampler																																		
			GC	Clayey gravels, gravel - sand - clay mixtures.		Pressure Meter																																		
	SANDS (More than 50% of coarse fraction is SMALLER than the No. 4 Sieve Size)	CLEAN SANDS (Little or no fines)	SW	Well graded sands, gravelly sands, little or no fines.	Water Table at time of drilling	Water Table after drilling																																		
		SANDS WITH FINES (Appreciable amount of fines)	SP	Poorly graded sands or gravelly sands, little or no fines.																																				
			SM	Silty sands, sand - silt mixtures																																				
			SC	Clayey sands, sand - clay mixtures.																																				
		FINE GRAINED SOILS (More than 50% of material is SMALLER than No. 200 sieve size)	SILTS AND CLAYS (Liquid limit LESS than 50)	ML	Inorganic silts and very fine sands, rock flour, silty of clayey fine sands or clayey silts and with slight plasticity.	Correlation of Penetration Resistance with Relative Density and Consistency <table border="1"> <thead> <tr> <th colspan="2">SAND & GRAVEL</th> <th colspan="2">SILT & CLAY</th> </tr> <tr> <th>No. of Blows</th> <th>Relative Density</th> <th>No. of Blows</th> <th>Consistency</th> </tr> </thead> <tbody> <tr> <td>0 - 4</td> <td>Very Loose</td> <td>0 - 1</td> <td>Very Soft</td> </tr> <tr> <td>5 - 10</td> <td>Loose</td> <td>2 - 4</td> <td>Soft</td> </tr> <tr> <td>11 - 30</td> <td>Medium Dense</td> <td>5 - 8</td> <td>Medium Stiff</td> </tr> <tr> <td>31 - 50</td> <td>Dense</td> <td>9 - 15</td> <td>Stiff</td> </tr> <tr> <td>Over 50</td> <td>Very Dense</td> <td>16 - 30</td> <td>Very Stiff</td> </tr> <tr> <td></td> <td></td> <td>Over 30</td> <td>Hard</td> </tr> </tbody> </table>			SAND & GRAVEL		SILT & CLAY		No. of Blows	Relative Density	No. of Blows	Consistency	0 - 4	Very Loose	0 - 1	Very Soft	5 - 10	Loose	2 - 4	Soft	11 - 30	Medium Dense	5 - 8	Medium Stiff	31 - 50	Dense	9 - 15	Stiff	Over 50	Very Dense	16 - 30	Very Stiff			Over 30	Hard
				SAND & GRAVEL					SILT & CLAY																															
No. of Blows	Relative Density			No. of Blows	Consistency																																			
0 - 4	Very Loose		0 - 1	Very Soft																																				
5 - 10	Loose		2 - 4	Soft																																				
11 - 30	Medium Dense		5 - 8	Medium Stiff																																				
31 - 50	Dense		9 - 15	Stiff																																				
Over 50	Very Dense	16 - 30	Very Stiff																																					
		Over 30	Hard																																					
CL	Inorganic lays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.																																							
OL	Organic silts and organic silty clays of low plasticity.																																							
SILTS AND CLAYS (Liquid limit GREATER than 50)	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts.																																						
	CH	Inorganic clays of high plasticity, fat clays																																						
	OH	Organic clays of medium to high plasticity, organic silts.																																						
	PT	Peat and other highly organic soils.																																						
HIGHLY ORGANIC SOILS																																								
BOUNDARY CLASSIFICATIONS: Soils possessing characteristics of two groups are designated by combinations of group symbols.																																								
<table border="1"> <tr> <td rowspan="2">SILT OR CLAY</td> <td colspan="3">SAND</td> <td colspan="2">GRAVEL</td> <td rowspan="2">Cobbles</td> <td rowspan="2">Boulders</td> </tr> <tr> <td>Fine</td> <td>Medium</td> <td>Coarse</td> <td>Fine</td> <td>Coarse</td> </tr> <tr> <td></td> <td>No.200</td> <td>No.40</td> <td>No.10 No.4</td> <td>3/4"</td> <td>3"</td> <td>12"</td> <td></td> </tr> </table> <p style="text-align: center;">U.S. STANDARD SIEVE SIZE</p>							SILT OR CLAY	SAND			GRAVEL		Cobbles	Boulders	Fine	Medium	Coarse	Fine	Coarse		No.200	No.40	No.10 No.4	3/4"	3"	12"														
SILT OR CLAY	SAND			GRAVEL		Cobbles		Boulders																																
	Fine	Medium	Coarse	Fine	Coarse																																			
	No.200	No.40	No.10 No.4	3/4"	3"	12"																																		
<h2>KEY TO SYMBOLS AND DESCRIPTIONS</h2>																																								
																																								

Reference: The Unified Soil Classification System, Corps of Engineers, U.S. Army Technical Memorandum No. 3-357, Vol. 1, March, 1953 (Revised April, 1960)

Figure A-2



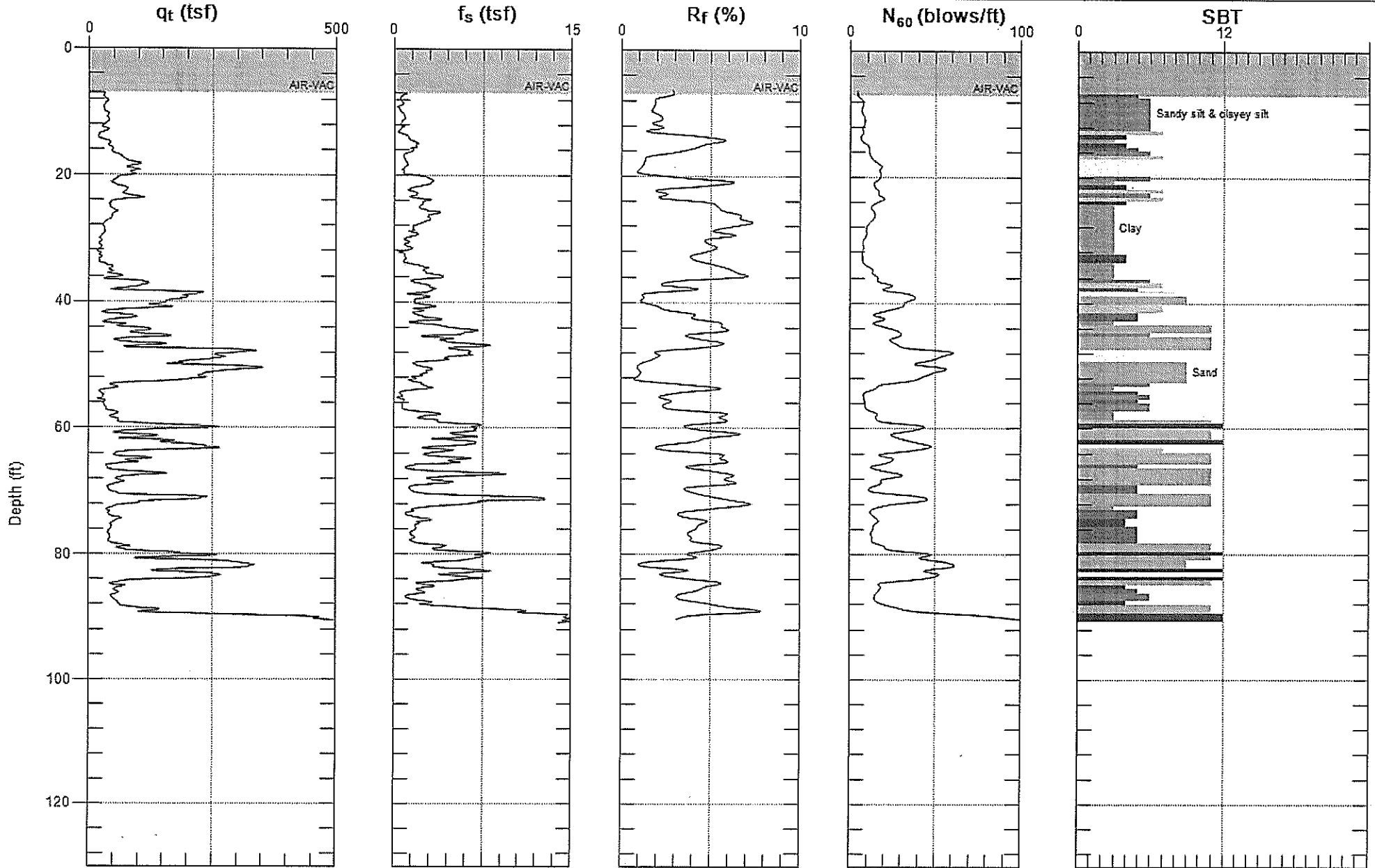
MACTEC

Site: MELROSE TRIANGLE

Engineer: W.CHAMBERLAIN

Sounding: CPT-01

Date: 8/19/2008 08:05



Max. Depth: 91.043 (ft)
Avg. Interval: 0.656 (ft)

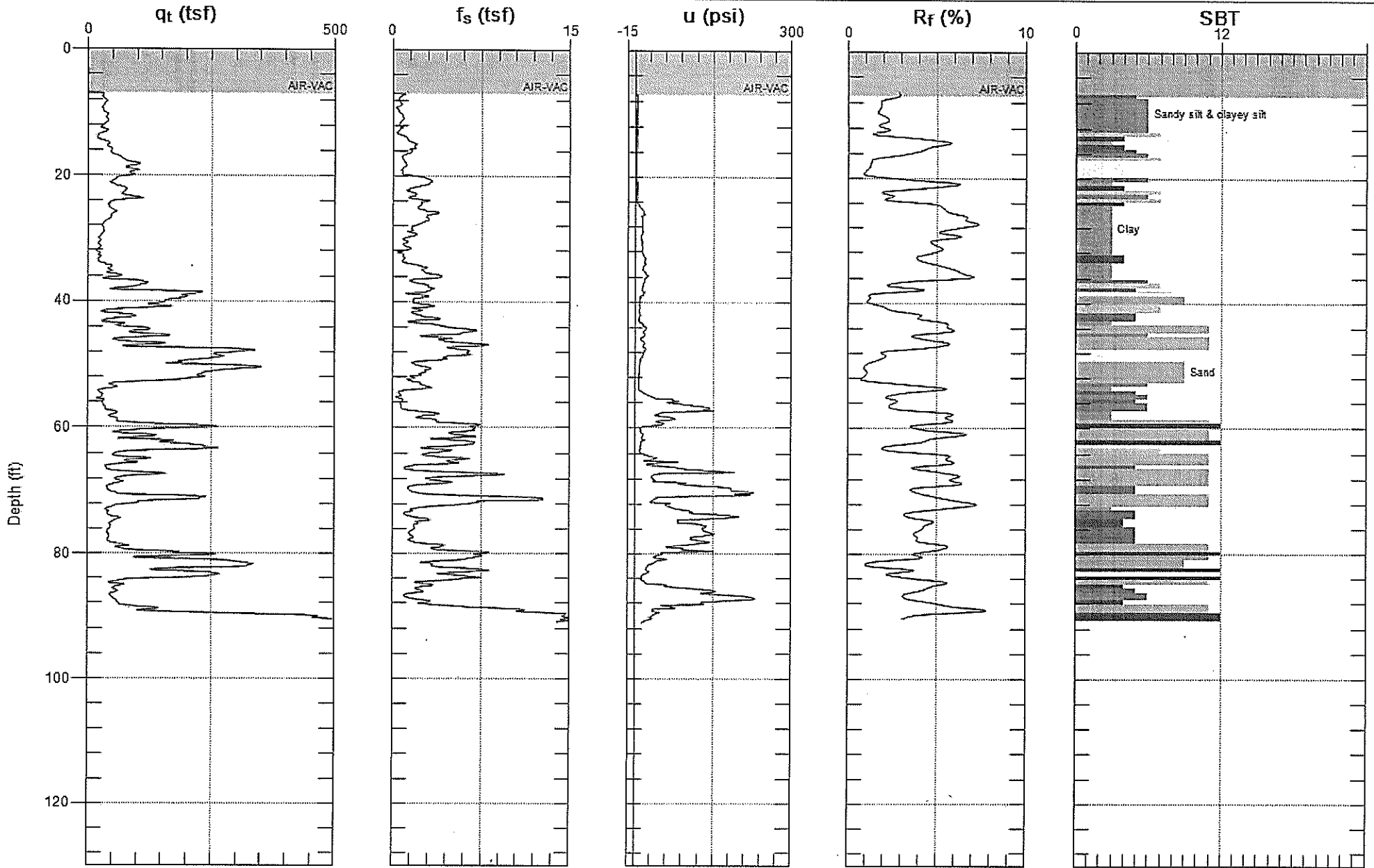
SBT: Soil Behavior Type (Robertson 1990)



MACTEC

Site: MELROSE TRIANGLE
Sounding: CPT-01

Engineer: W.CHAMBERLAIN
Date: 8/19/2008 08:05



Max. Depth: 91.043 (ft)
Avg. Interval: 0.656 (ft)

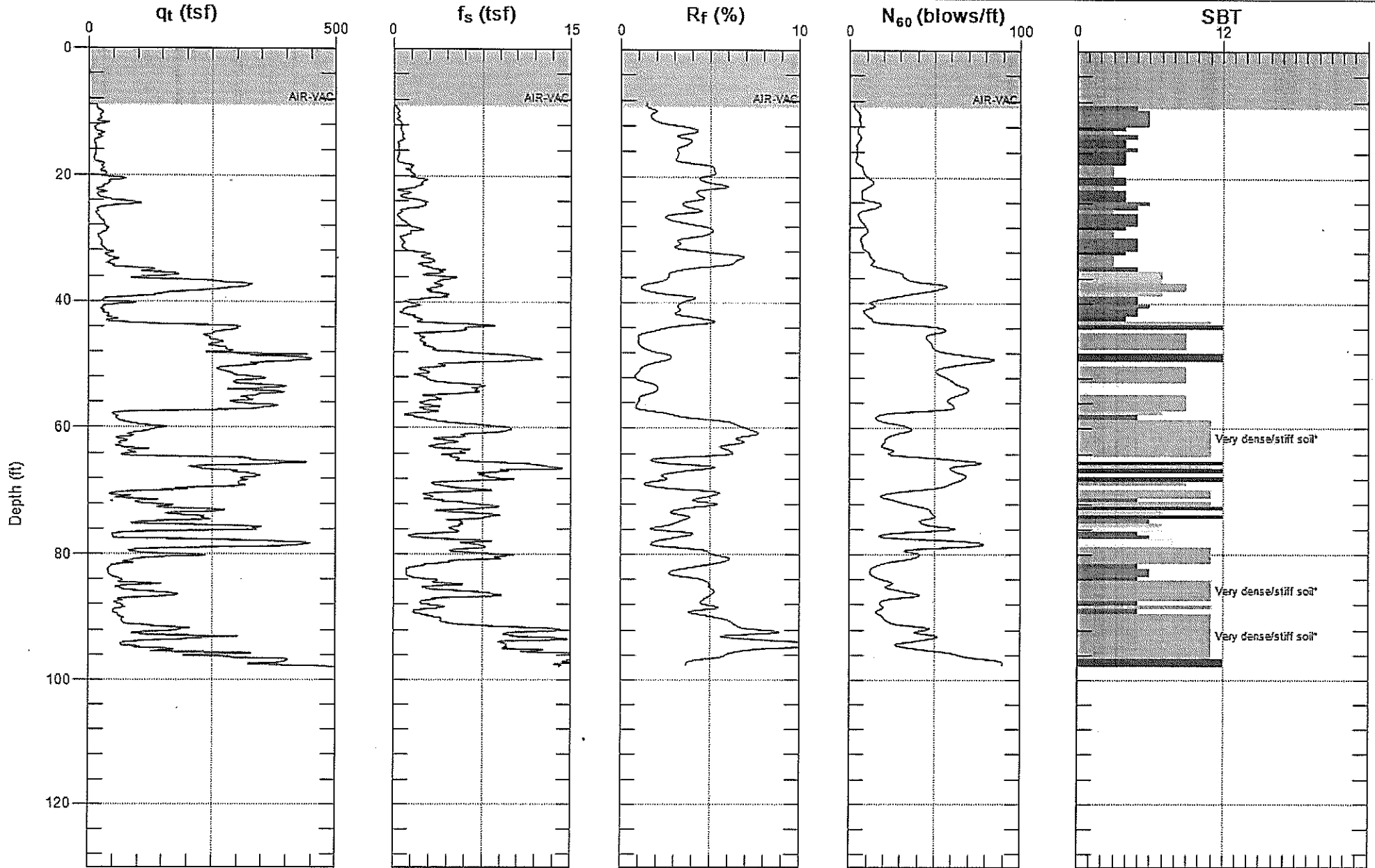
SBT: Soil Behavior Type (Robertson 1990)



MACTEC

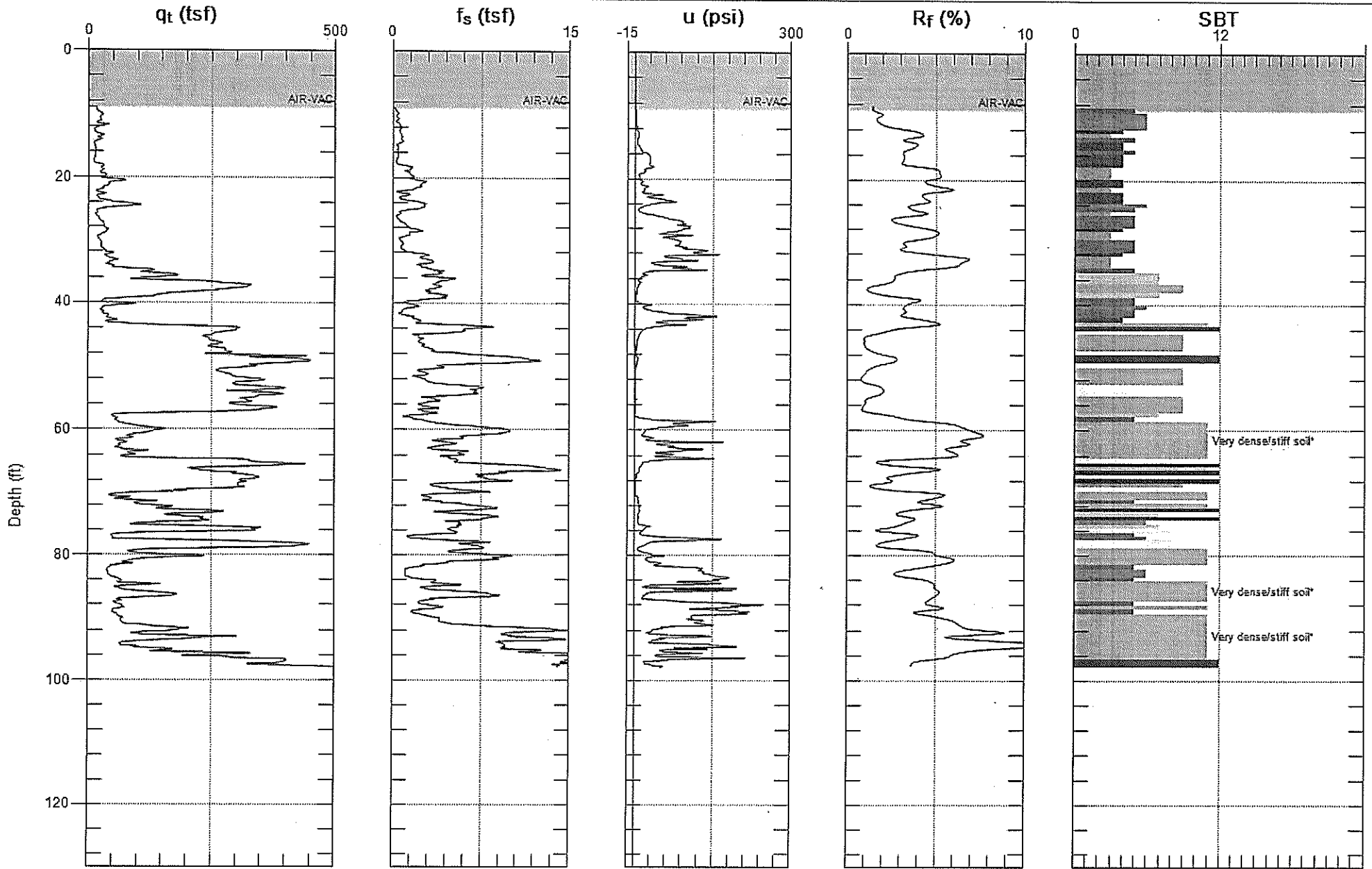
Site: MELROSE TRIANGLE
Sounding: CPT-02

Engineer: W.CHAMBERLAIN
Date: 8/19/2008 11:18



Max. Depth: 97.933 (ft)
Avg. Interval: 0.656 (ft)

SBT: Soil Behavior Type (Robertson 1990)



Max. Depth: 97.933 (ft)
 Avg. Interval: 0.656 (ft)

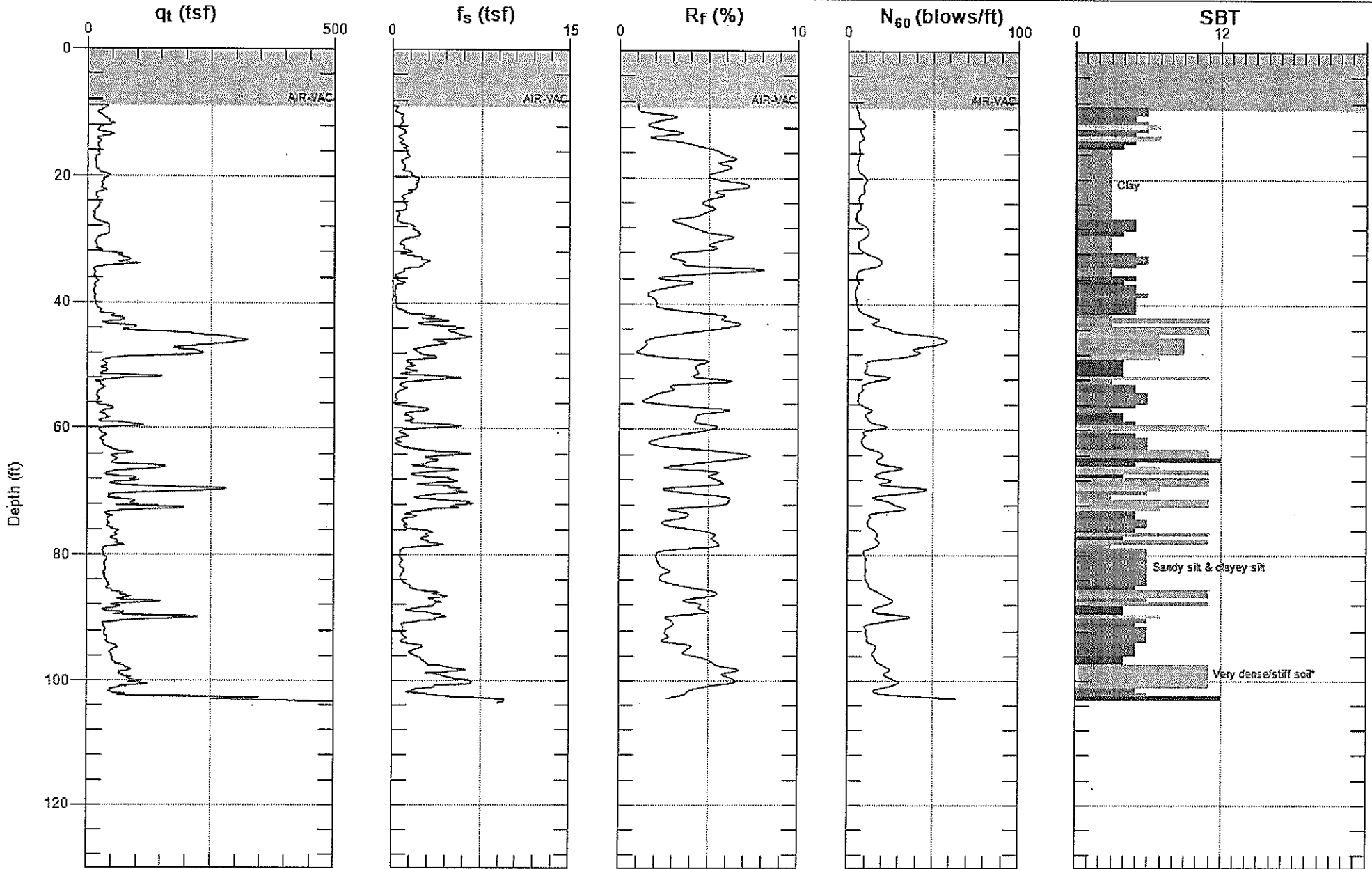
SBT: Soil Behavior Type (Robertson 1990)



MACTEC

Site: MELROSE TRIANGLE
Sounding: CPT-03

Engineer: W.CHAMBERLAIN
Date: 8/19/2008 10:25



Max. Depth: 103.675 (ft)
Avg. Interval: 0.656 (ft)

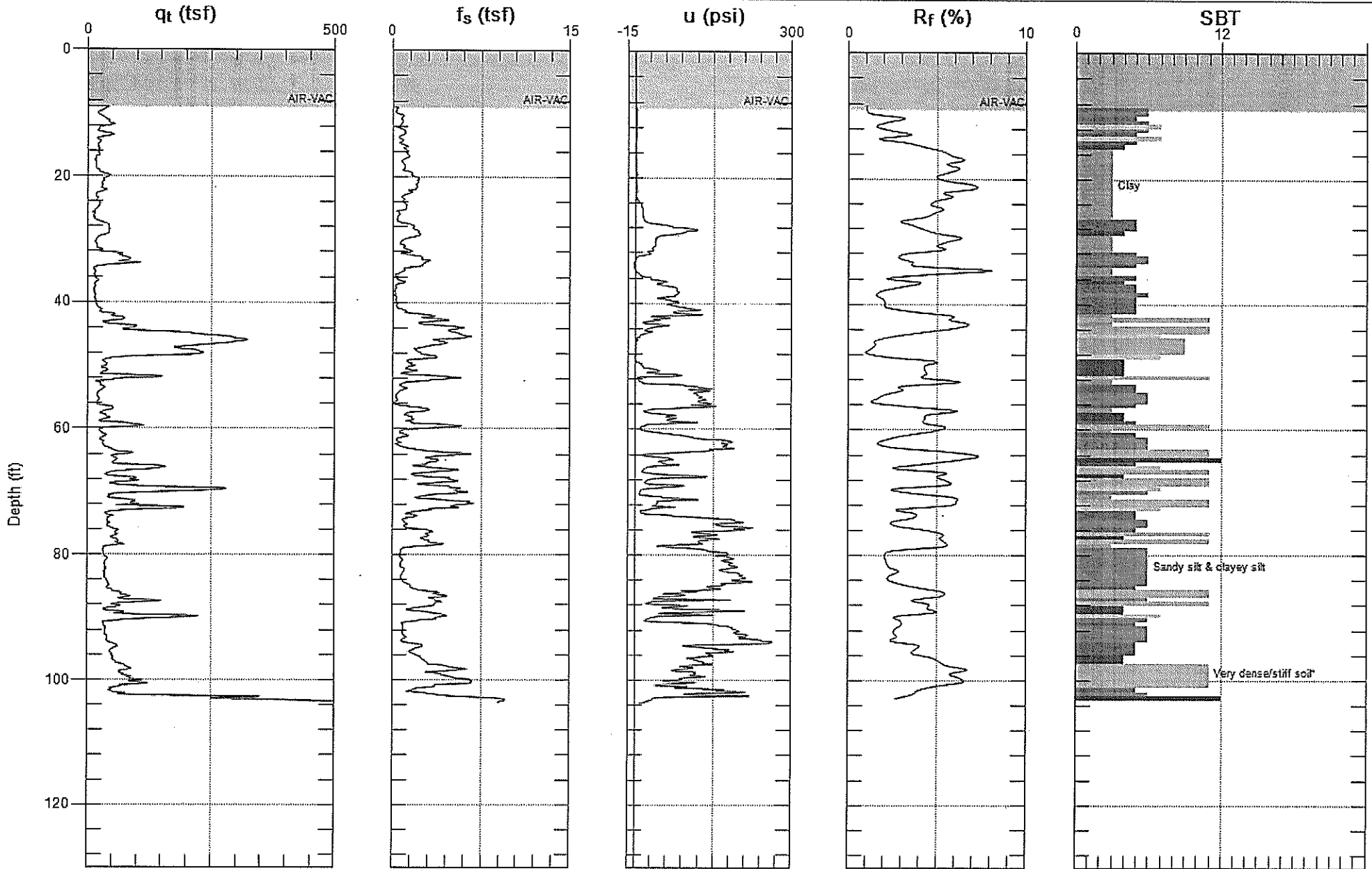
SBT: Soil Behavior Type (Robertson 1990)



MACTEC

Site: MELROSE TRIANGLE
Sounding: CPT-03

Engineer: W.CHAMBERLAIN
Date: 8/19/2008 10:25



Max. Depth: 103.675 (ft)
Avg. Interval: 0.656 (ft)

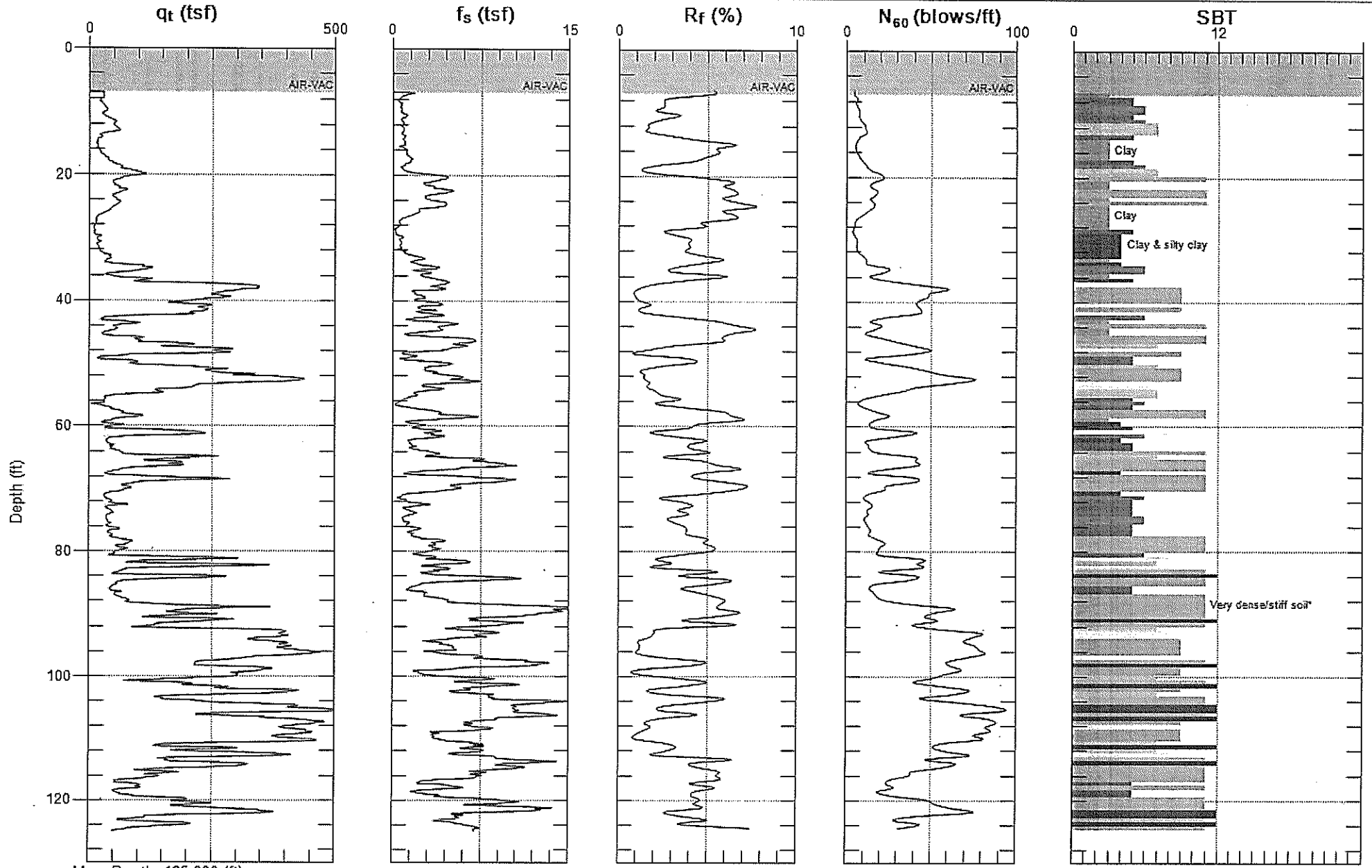
SBT: Soil Behavior Type (Robertson 1990)



MACTEC

Site: MELROSE TRIANGLE
Sounding: CPT-04

Engineer: W.CHAMBERLAIN
Date: 8/19/2008 09:08



Max. Depth: 125.000 (ft)
Avg. Interval: 0.656 (ft)

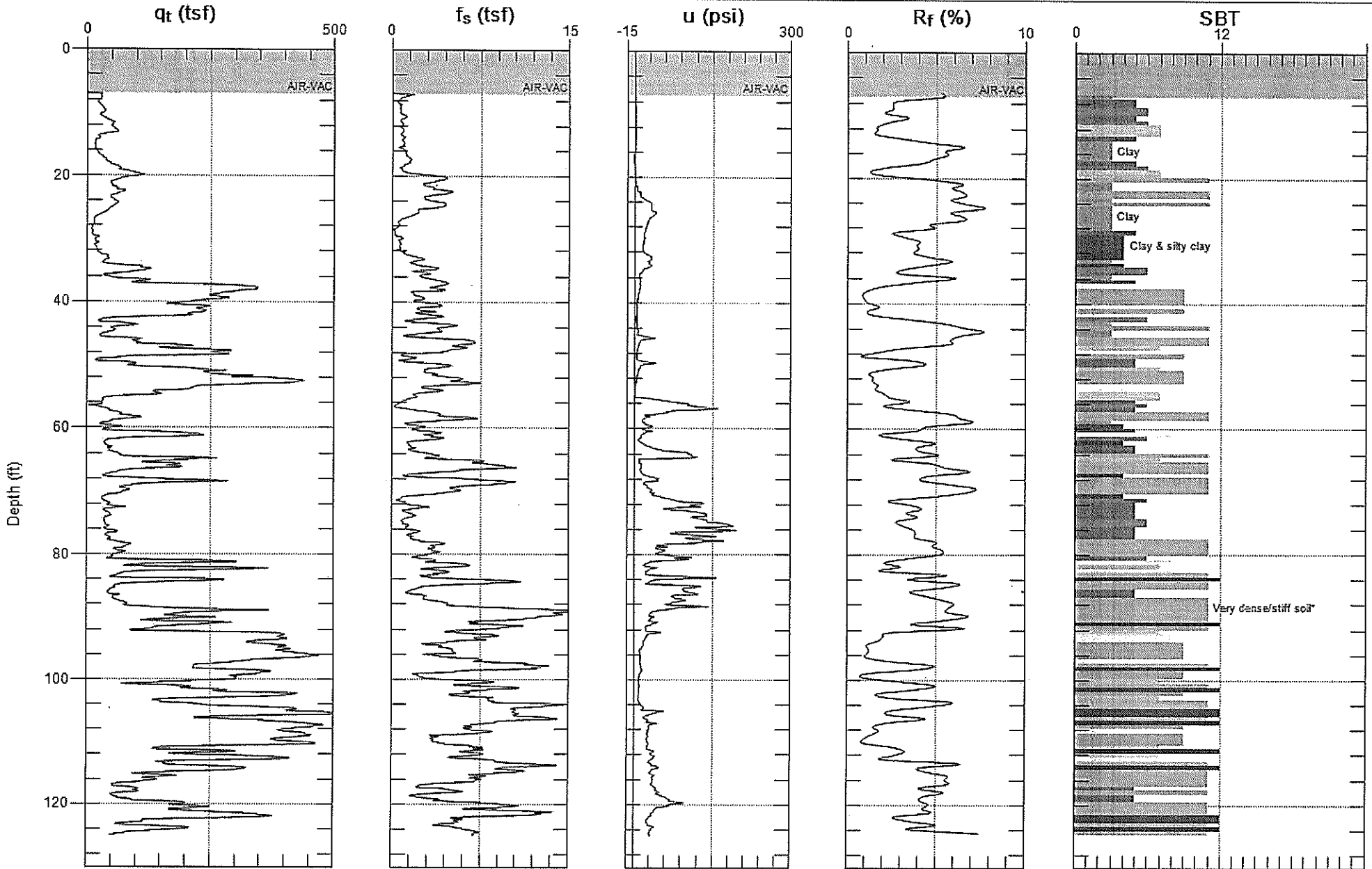
SBT: Soil Behavior Type (Robertson 1990)



MACTEC

Site: MELROSE TRIANGLE
Sounding: CPT-04

Engineer: W.CHAMBERLAIN
Date: 8/19/2008 09:08



Max. Depth: 125.000 (ft)
Avg. Interval: 0.656 (ft)

SBT: Soil Behavior Type (Robertson 1990)

PSOMAS

Balancing the Natural and Built Environment

January 8, 2009

VIA EMAIL

wbchamberlain@mactec.com

Mr. Warren B. Chamberlain, R.G., C.Hg., P.E.
MACTEC Engineering and Consulting, Inc.
600 Grand Avenue Suite 300
Oakland, California 94610
Phone: 510.628.3228

Subject: Melrose Triangle Monitor Well Locations, West Hollywood
Psomas Job No. IMAC050301

Dear Warren:

The locations of the monitor wells for the above project site are shown below as well as the datums upon which they were based.

Well No.	Northing (feet)	Easting (feet)	Elevation (feet)	Latitude	Longitude	Description
EW-001	1852087.82	6443935.60	219.84	34-04-51.60482	118-23-19.67504	Top 6" dia PVC N edge
OB-001	1852059.86	6443757.02	224.21	34-04-51.32143	118-23-21.79681	Top 2" dia PVC N edge
OB-002	1852067.79	6444245.59	211.72	34-04-51.41858	118-23-15.98874	Top 2" dia PVC N edge
OB-002	1852067.63	6444245.67	211.75	34-04-51.41693	118-23-15.98785	Top 2" dia PVC S edge
OB-003	1852443.91	6444236.49	221.83	34-04-55.13882	118-23-16.11422	Top 2" dia PVC N edge

Datum information:

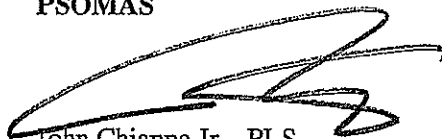
Horizontal: Reference Frame: NAD83 (cors96) (epoch:2002.0000), State Plane Coordinates (0405 CA 5) California Zone 5 - NAD83 values are taken from GPS observations and field survey methods performed on 09/23/2008 with values obtained from OPUS processing on 10/02/2008.

Vertical Bench Marks:

County of Los Angeles Bench Mark No Y 5322, 2003 Adjustment NAVD88, Fairfax Quad, Elevation = 69.800 meters (229.003 feet), Description - cut spk n cb Santa Monica Blvd (n barrel) 14m n/o c/l & 25.5m w/o c/l Doheny Dr @ e end cb

County of Los Angeles Bench Mark No Y 11596, 2003 Adjustment NAVD88, Fairfax Quad, Elevation = 68.489 meters (224.702 feet), Description - LACO BM tag in w cb Doheny Dr 1m s/o bcr 7m w/o c/l & 29m s/o c/l Santa Monica Blvd (s barrel)

Sincerely,
PSOMAS



John Chiappe Jr., PLS
Senior Project Manager



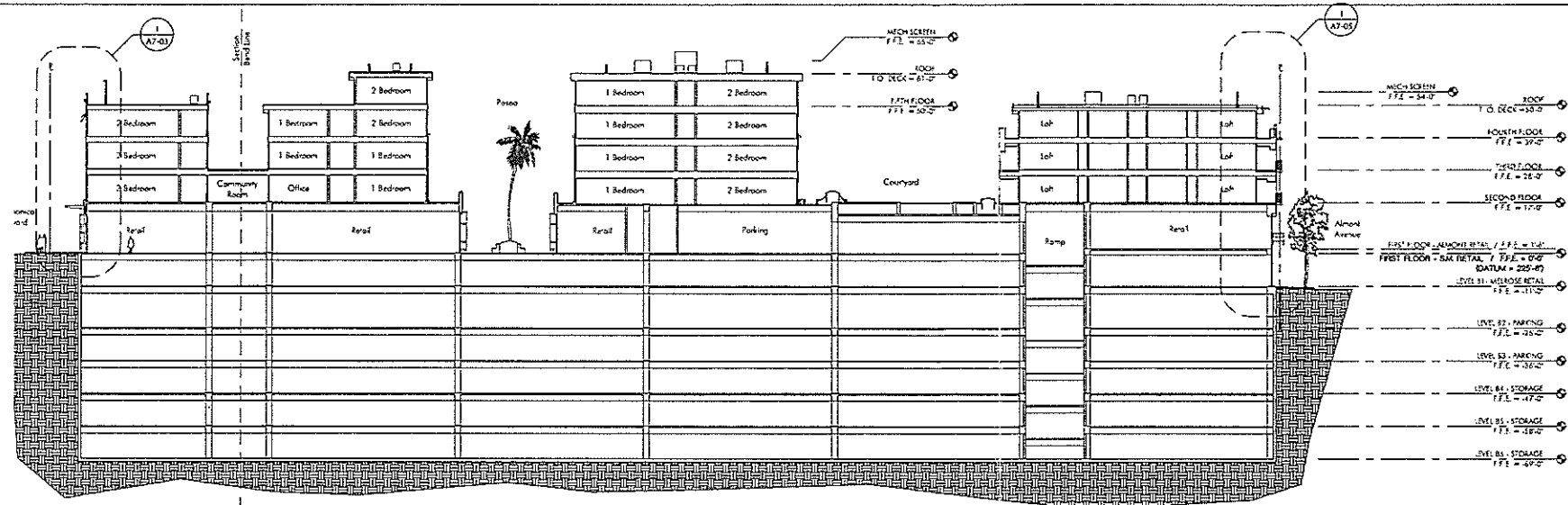
555 South Flower Street
Suite 4400
Los Angeles, CA 90071

P:213.223.1400
F:213.223.1444
www.psomas.com

1mac050301_mon_well_cert_letter_mactec_chamberlain

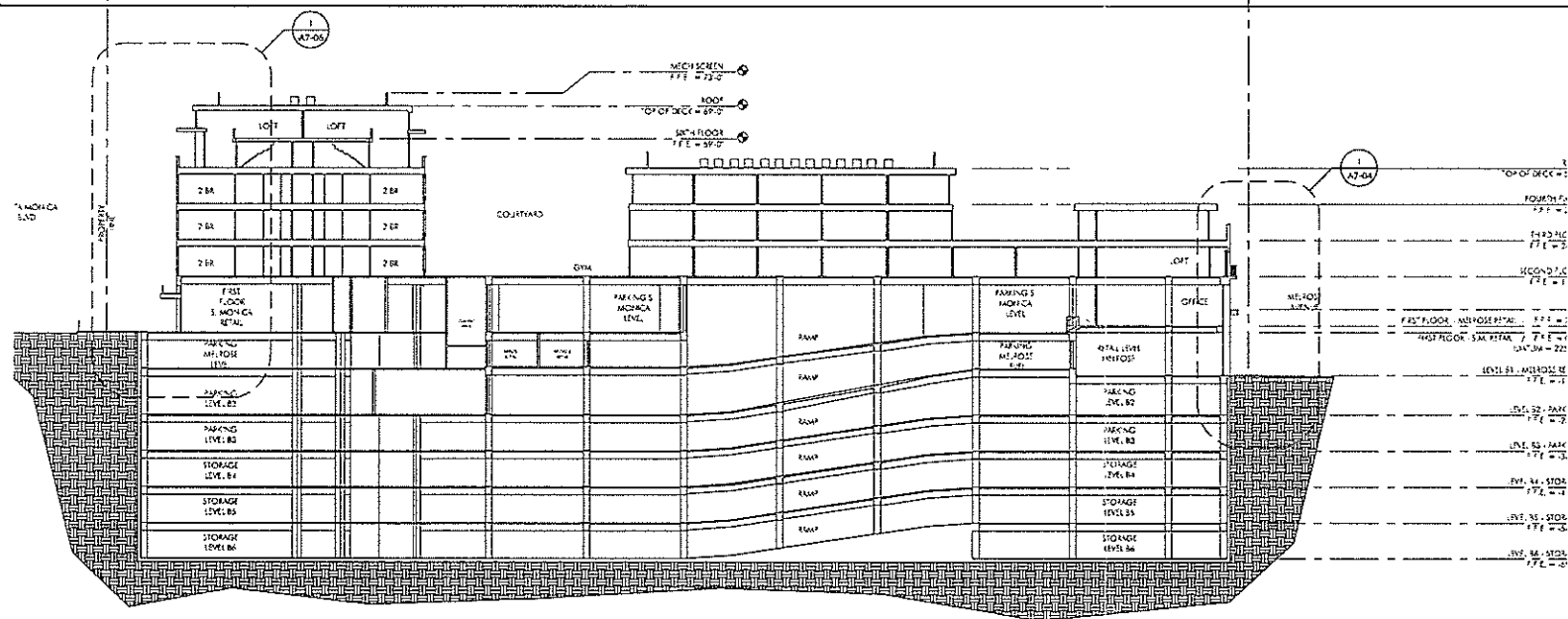
APPENDIX C

SITE ELEVATION PLANS

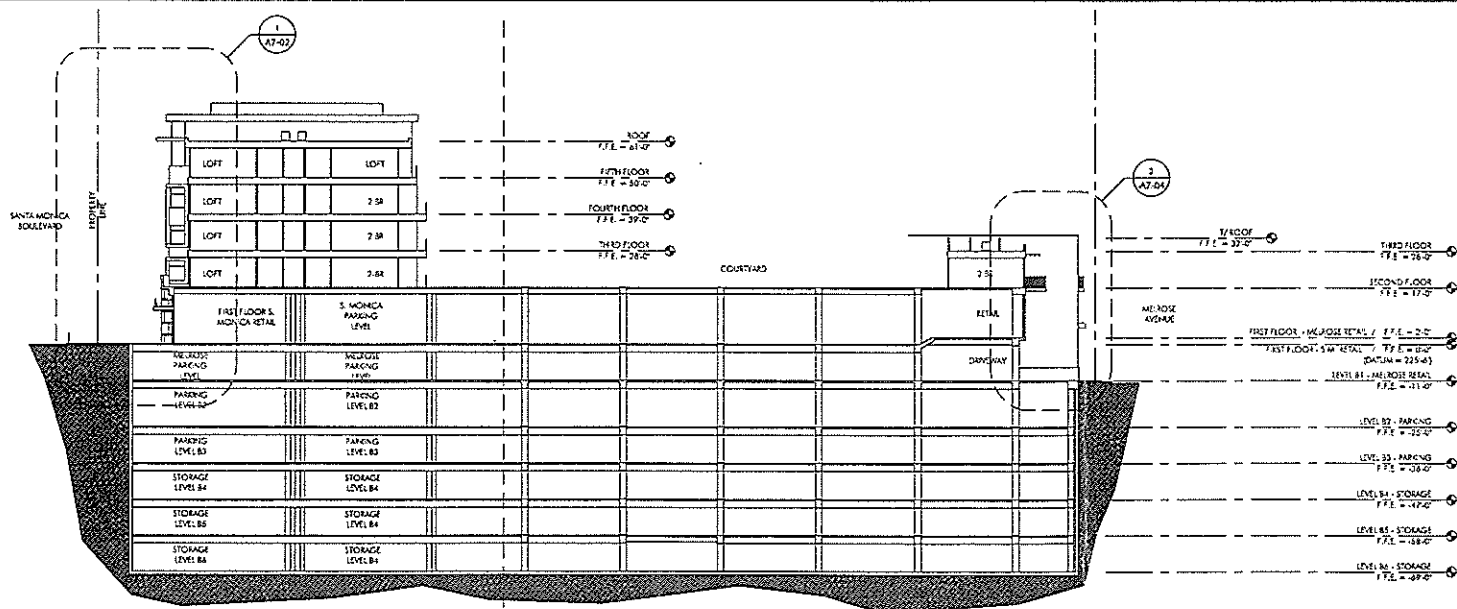


SECTION 1/A .03

1

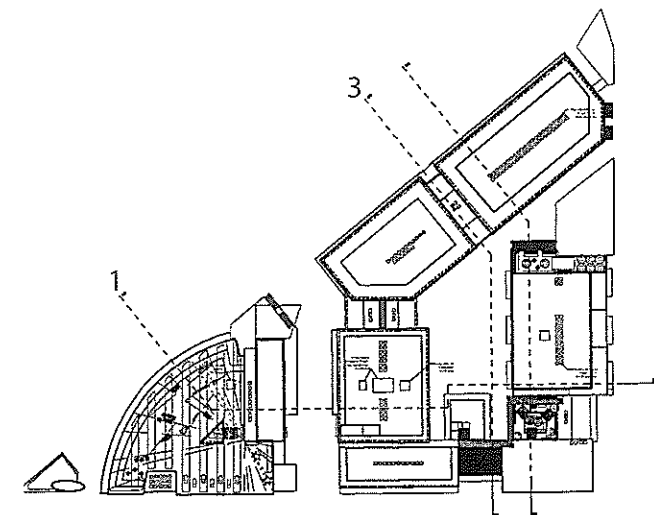


SECTION 1/A .01



SECTION 1/A .01

3



SECTION LOCATION

MELROSE TRIANGLE
WEST HOLLYWOOD,
CALIFORNIA

#	Date	Issue
1	02.04.01	Development Permit
2	06.27.03	Progress Plan
3	07.11.03	Development Amendment
4	11.08.04	City Submittal
5	12.09.04	DR Presentation
6	08.31.06	City Design Review Submittal

Job No. 01.234.11

Date: 08.31.2006
Scale: no scale

SITE SECTIONS

A6.01

NOT ISSUED FOR CONSTRUCTION

APPENDIX D

PUMP TEST TIME-DRAWDOWN PLOTS

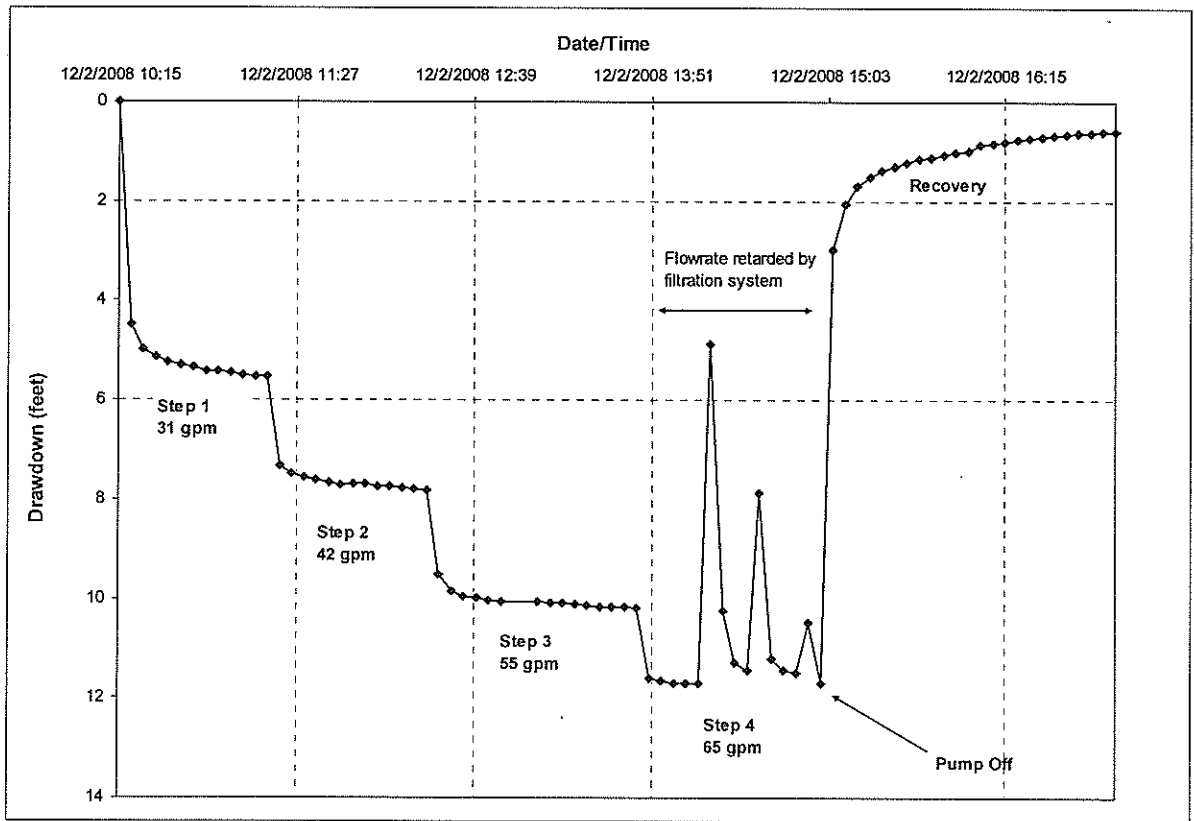


Figure D-1: Step Test Drawdown Data

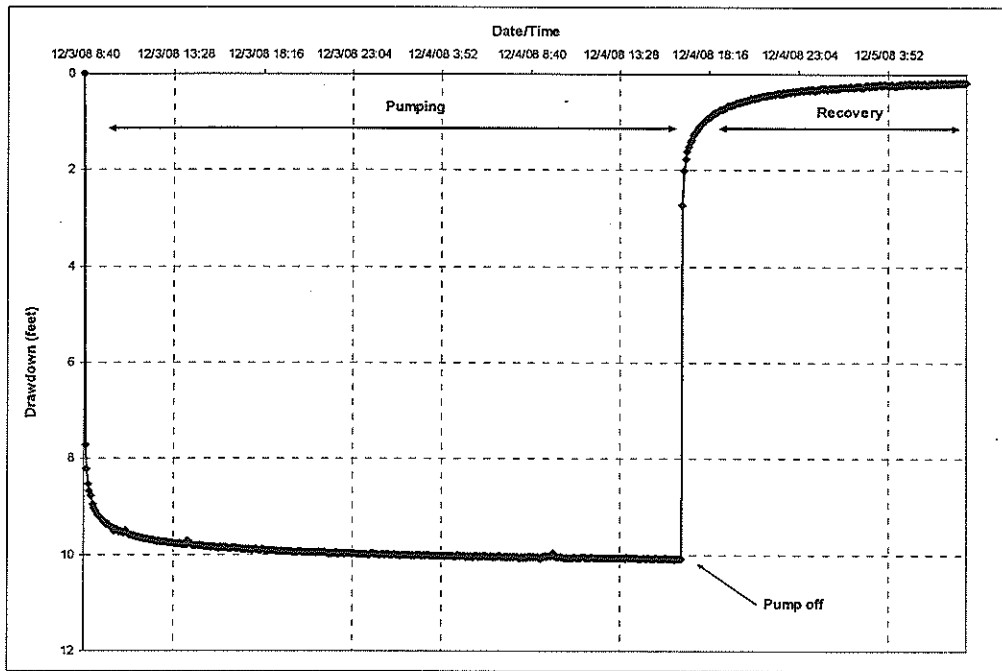


Figure D-2: Drawdown observations at EW-1 for the constant rate pumping test

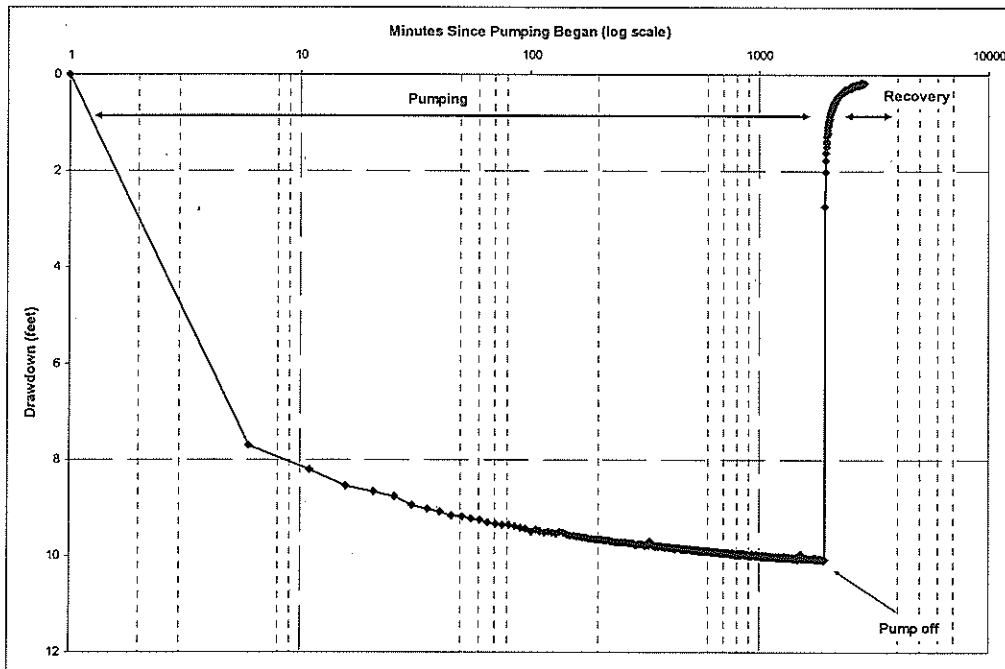


Figure D-3: Drawdown vs. Time at EW-1 for the constant rate pumping test (semi-log plot)

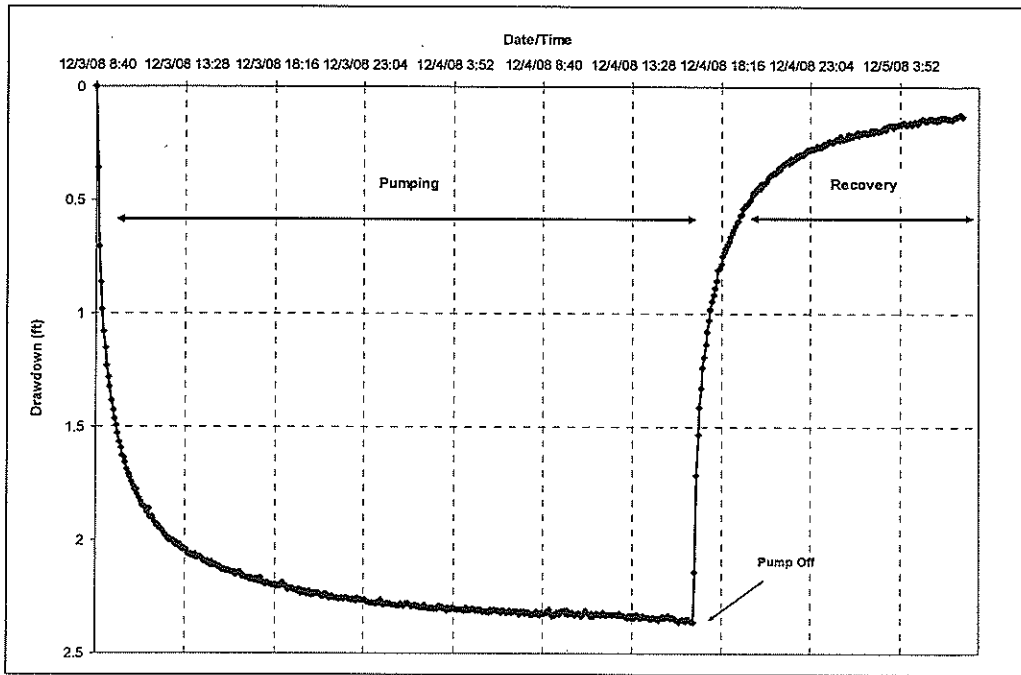


Figure D-4: Drawdown observations at OB-1 for the constant rate pumping test

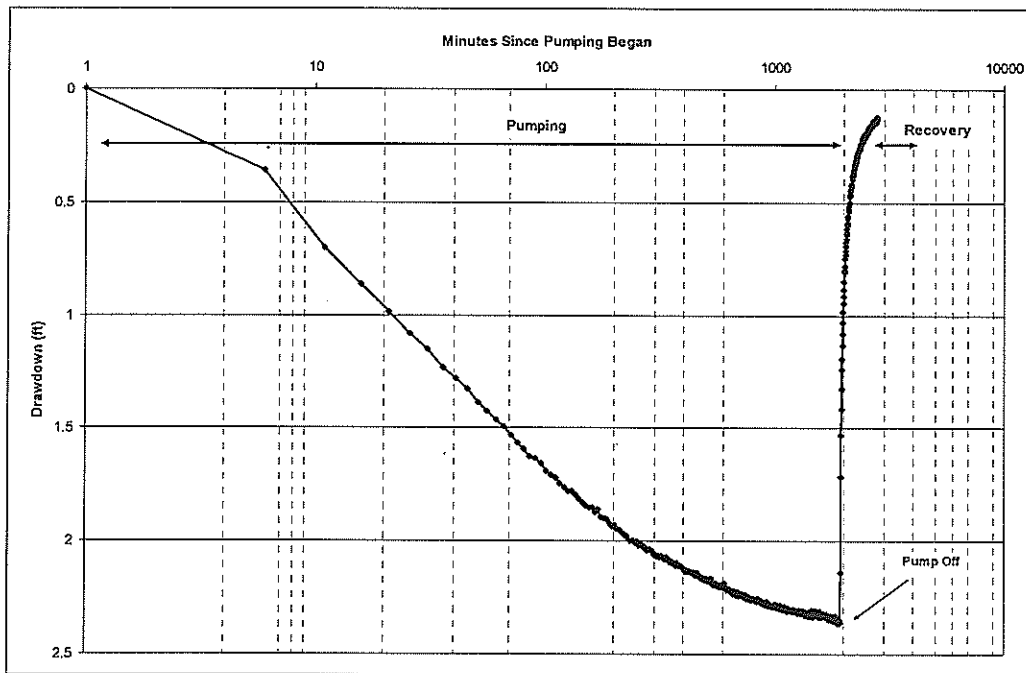


Figure D-5: Drawdown vs. Time at OB-1 for the constant rate pumping test (semi-log plot)

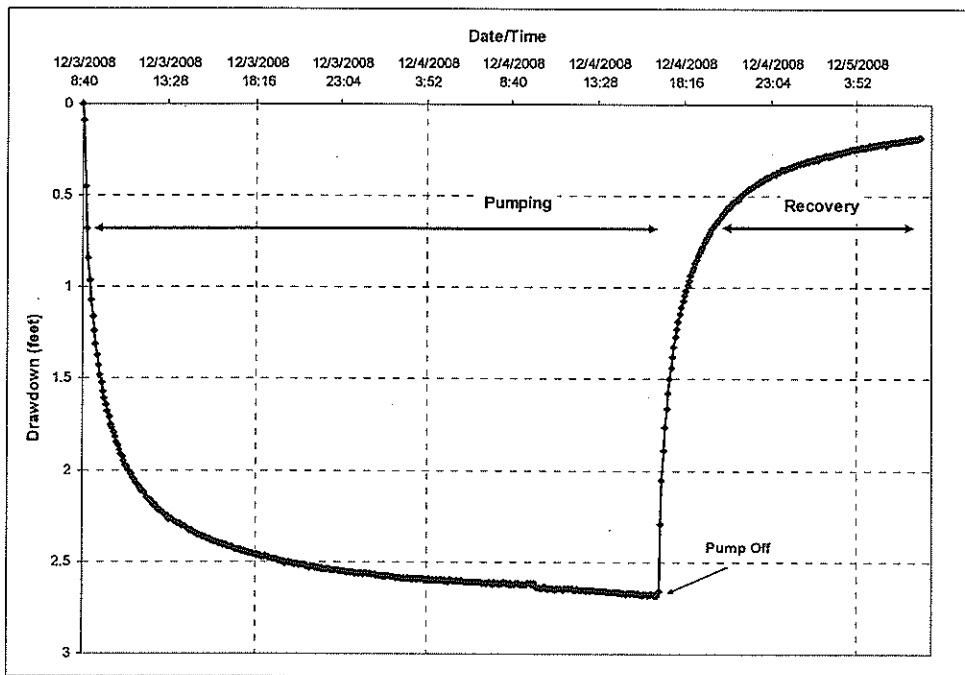


Figure D-6: Drawdown observations at OB-2 for the constant rate pumping test

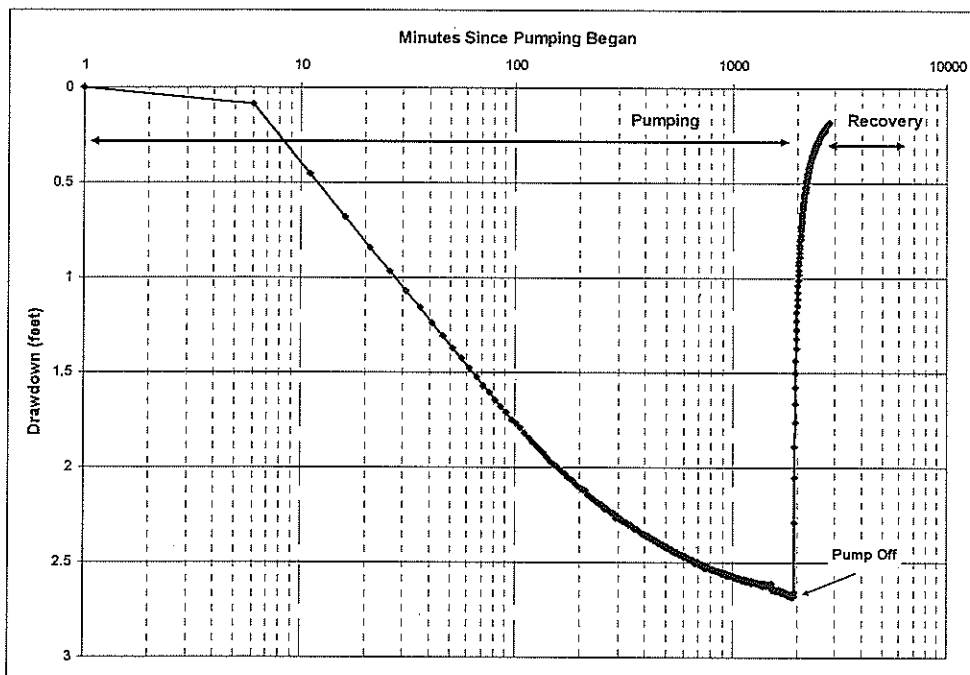


Figure D-7: Drawdown vs. Time at OB-2 for the constant rate pumping test (semi-log plot)

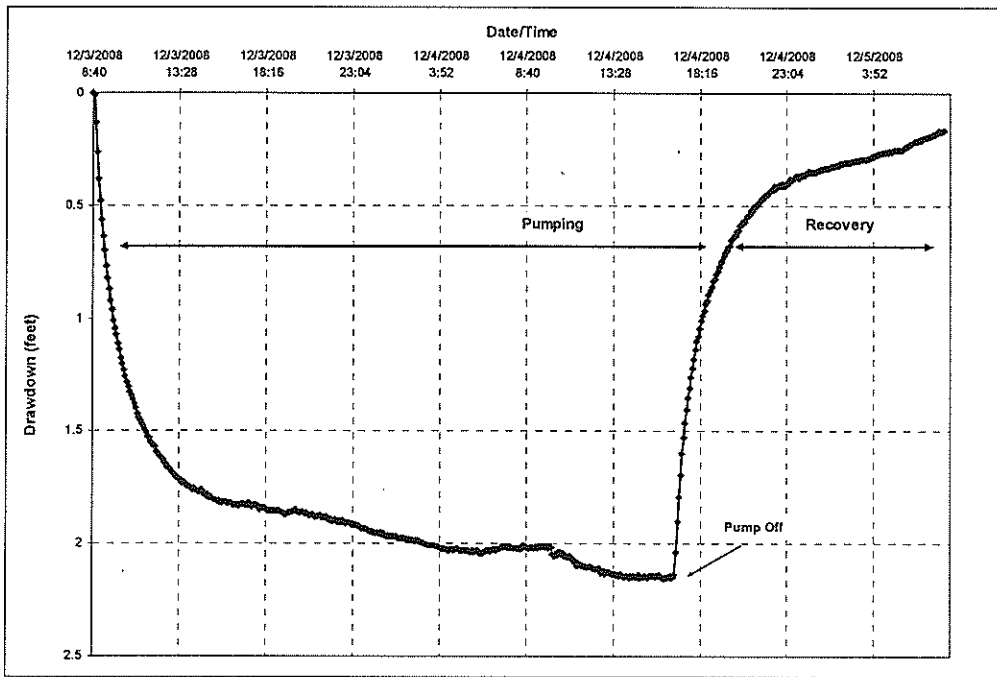


Figure D-8: Drawdown observations at OB-3 for the constant rate pumping test

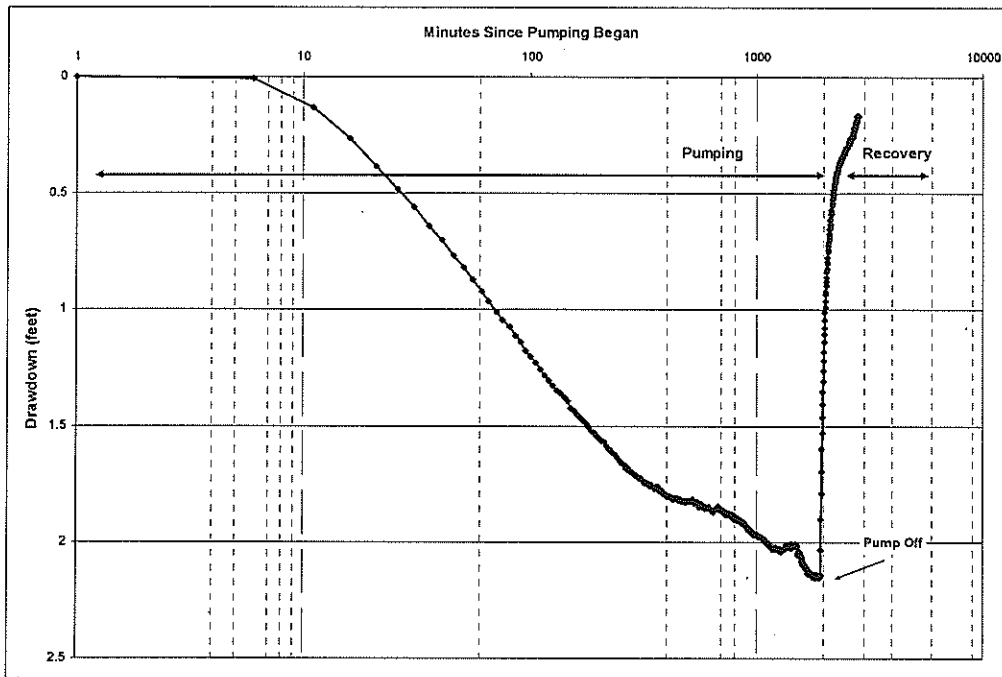


Figure D-9: Drawdown vs. Time at OB-3 for the constant rate pumping test (semi-log plot)

APPENDIX E

CURVE MATCHING PLOTS FOR PUMP TEST DATA

STEP DRAWDOWN TEST

Data Set: C:\...\steptest_wbc.aqt
 Date: 01/02/09 Time: 17:12:46

PROJECT INFORMATION

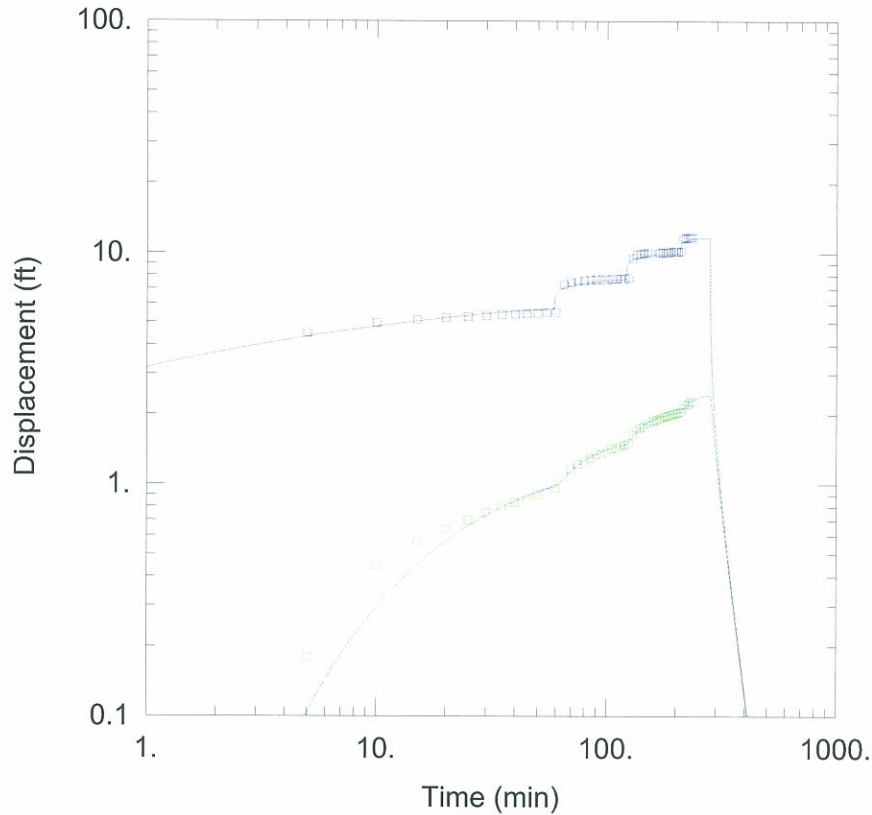
Company: MACTEC
 Client: SYSTEM LLC
 Project: 4088087537
 Location: Melrose Triangle
 Test Well: EW-1
 Test Date: Dec 2008

SOLUTION

Aquifer Model: Leaky
 Solution Method: Hantush-Jacob

T = 640. ft²/day
 S = 0.0003406
 1/B = 0.0032 ft⁻¹
 Sw = -3.
 C = 0.7 min²/ft⁵
 P = 2.639E-308

Step Test Model: Jacob-Rorabaugh
 Time (t) = 1. min Rate (Q) in cu. ft/min
 s(t) = 0.6009Q + 0.7Q^{2.639E-308}
 W.E. = 1.112E+4% (Q from last step)



AQUIFER DATA

Saturated Thickness: 70. ft

Anisotropy Ratio (Kz/Kr): 1.

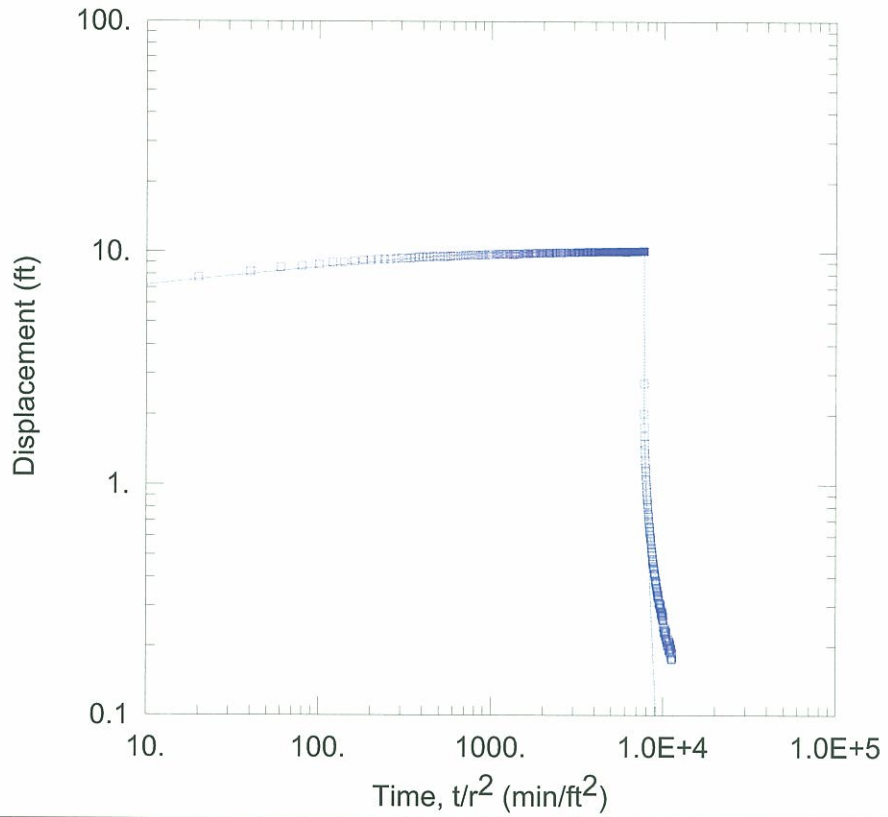
WELL DATA

Pumping Wells

Well Name	X (ft)	Y (ft)
EW-1	6443935.6	1852087.82

Observation Wells

Well Name	X (ft)	Y (ft)
EW-1	6443935.6	1852087.82
OB-1	6443757.02	1852059.86



CONSTANT RATE PUMP TEST

Data Set: C:\...\MT_EW1Hantush-rec_wbc.aqt
 Date: 01/02/09 Time: 17:15:06

PROJECT INFORMATION

Company: MACTEC
 Client: SYSTEM LLC
 Project: 4088087537
 Location: Melrose Traingle
 Test Well: EW-1
 Test Date: Dec 2008

SOLUTION

Aquifer Model: Leaky
 Solution Method: Hantush
 T = 653.7 ft²/day
 S = 0.0006187
 r/B' = 0.01778
 β' = 0.1
 r/B'' = 0.01514
 β'' = 0.

AQUIFER DATA

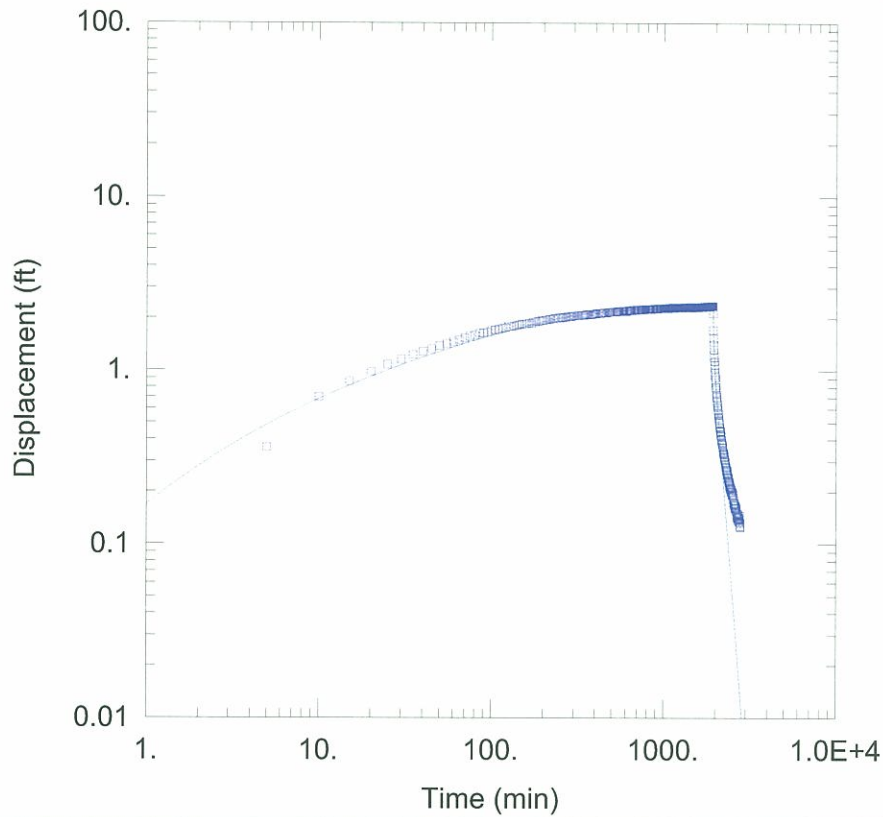
Saturated Thickness: 70. ft

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA

Pumping Wells		
Well Name	X (ft)	Y (ft)
EW-1 (recovery)	0	0

Observation Wells		
Well Name	X (ft)	Y (ft)
EW-1 (recovery)	0	0



OBSERVATION WELL OB-1

Data Set: C:\...\lob1hantush_wbc.aqt
 Date: 01/02/09 Time: 17:33:22

PROJECT INFORMATION

Company: MACTEC
 Client: SYSTEM LLC
 Project: 4088087537
 Location: Melrose Triangle
 Test Well: EW-1
 Test Date: Dec 2008

SOLUTION

Aquifer Model: Leaky
 Solution Method: Hantush
 T = 652.1 ft²/day
 S = 7.311E-7
 1/B' = 0.002733 ft⁻¹
 B'/r = 0.03858 ft⁻¹
 1/B'' = 0.1 ft⁻¹
 B''/r = 0. ft⁻¹

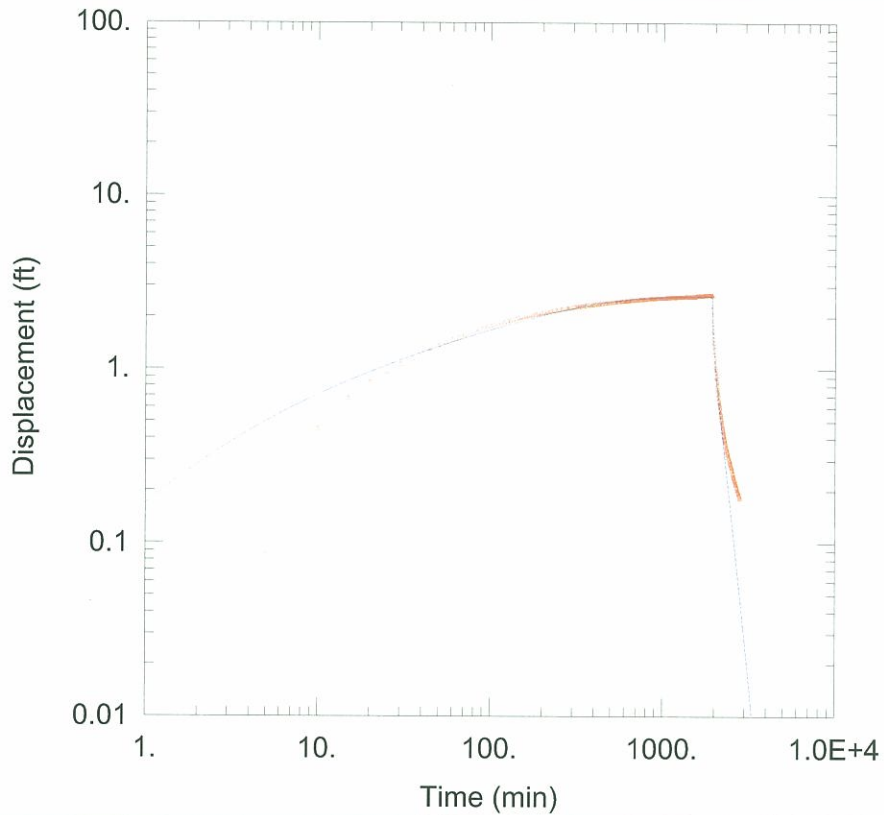
AQUIFER DATA

Saturated Thickness: 70. ft

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA

Pumping Wells			Observation Wells		
Well Name	X (ft)	Y (ft)	Well Name	X (ft)	Y (ft)
EW-1	6443935.6	1852087.82	OB-1	6443757.02	1852059.86



OBSERVATION WELL OB-2

Data Set: C:\...\lob2hantush-wbc.aqt
 Date: 01/02/09 Time: 17:11:00

PROJECT INFORMATION

Company: MACTEC
 Client: SYSTEM LLC
 Project: 4088087537
 Location: Melrose Triangle
 Test Well: EW-1
 Test Date: dec 2008

SOLUTION

Aquifer Model: Leaky
 Solution Method: Hantush
 T = 609. ft²/day
 S = 7.692E-7
 1/B' = 0.001464 ft⁻¹
 B'/r = 0.01267 ft⁻¹
 1/B'' = 0.05 ft⁻¹
 B''/r = 0. ft⁻¹

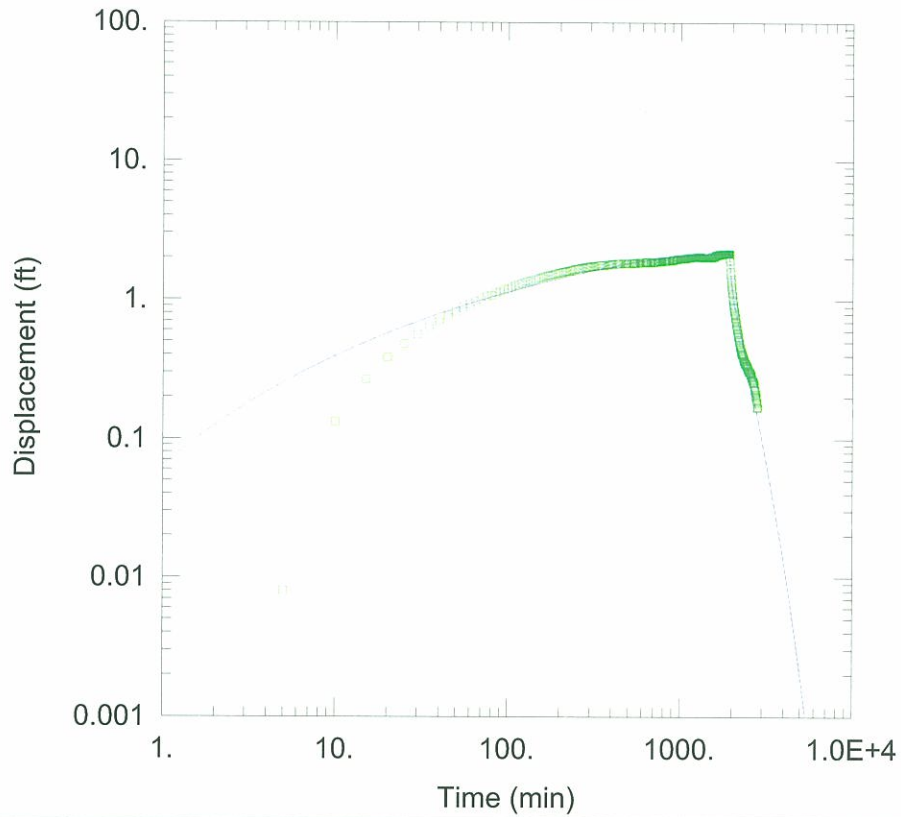
AQUIFER DATA

Saturated Thickness: 70. ft

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA

Pumping Wells			Observation Wells		
Well Name	X (ft)	Y (ft)	Well Name	X (ft)	Y (ft)
EW-1	6443935.6	1852087.82	OB-2	6444245.59	1852067.79



OBSERVATION WELL OB-3

Data Set: C:\...\lob3hantush_wbc.aqt
 Date: 01/02/09 Time: 17:30:09

PROJECT INFORMATION

Company: MACTEC
 Client: SYSTEM LLC
 Project: 4088087537
 Location: Melrose Triangle
 Test Well: EW-1
 Test Date: Dec 2008

SOLUTION

Aquifer Model: Leaky
 Solution Method: Hantush
 $T = 653. \text{ ft}^2/\text{day}$
 $S = 1.253\text{E-}7$
 $1/B' = 0.001113 \text{ ft}^{-1}$
 $\beta'/r = 0.02393 \text{ ft}^{-1}$
 $1/B'' = 10.8 \text{ ft}^{-1}$
 $\beta''/r = 0. \text{ ft}^{-1}$

AQUIFER DATA

Saturated Thickness: 70. ft

Anisotropy Ratio (K_z/K_r): 1.

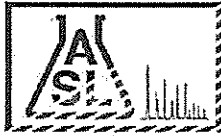
WELL DATA

Pumping Wells			Observation Wells		
Well Name	X (ft)	Y (ft)	Well Name	X (ft)	Y (ft)
EW-1	6443935.6	1852087.82	OB-3	6444236.49	1852443.91

APPENDIX F

LABORATORY ANALYTICAL DATA SHEETS AND CHAIN-OF-CUSTODY

DOCUMENTATION



AMERICAN SCIENTIFIC LABORATORIES, LLC
Environmental Testing Services

2520 N. San Fernando Rd., Los Angeles, CA 90065 Tel: (323) 223-9700 Fax: (323) 223-9500

ANALYTICAL RESULTS

Ordered By

Mactec
 600 Grand Ave. Suite # 300
 Oakland, CA 94610-

Site

9037 Melrose Ave.
 West Hollywood, CA 90069

Telephone: (510)628-3228

Attn: Warren Chamberlain

Page: 2

Project ID: 4088.08.7937.02
 Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 1664, Revision A, Oil and Grease (HEM)

QC Batch No: 100108-2

Our Lab I.D.			225768			
Client Sample I.D.			EW-1			
Date Sampled			10/01/2008			
Date Prepared			10/02/2008			
Preparation Method						
Date Analyzed			10/02/2008			
Matrix			Water			
Units			mg/L			
Dilution Factor			1			
Analytes	MDL	PQL	Results			
Conventionals						
Oil and Grease	2.69	5.00	ND			

QUALITY CONTROL REPORT

QC Batch No: 100108-2

Analytes	LCS % REC	LCS/LCSD % Limit							
Conventionals									
Oil and Grease	98	80-120							



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 Oakland, CA 94610-

Site

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 West Hollywood, CA 90069

Telephone: (510)628-3228

Attn: Warren Chamberlain

Page: 3

Project ID: 4088.08.7937.02
 Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 180.1, Turbidity (Nephelometric)

QC Batch No: 100108-2

Our Lab I.D.			225768			
Client Sample I.D.			EW-1			
Date Sampled			10/01/2008			
Date Prepared			10/01/2008			
Preparation Method						
Date Analyzed			10/01/2008			
Matrix			Water			
Units			NTU			
Dilution Factor			1			
Analytes	MDL	PQL	Results			
Conventionals						
Turbidity	0.0100	1.00	ND			

QUALITY CONTROL REPORT

QC Batch No: 100108-2

Analytes	LCS % REC	LCS/LCSD % Limit							
Conventionals									
Turbidity	100	80-120							



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Site

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 West Hollywood, CA 90069

Telephone: (510)628-3228

Attn: Warren Chamberlain

Page: 4

Project ID: 4088.08.7937.02
 Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 218.6, Hexavalent Chromium by Ion Chromatography

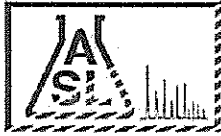
QC Batch No: 100108-2

Our Lab I.D.			225768			
Client Sample I.D.			EW-1			
Date Sampled			10/01/2008			
Date Prepared			10/02/2008			
Preparation Method						
Date Analyzed			10/02/2008			
Matrix			Water			
Units			ug/L			
Dilution Factor			1			
Analytes	MDL	PQL	Results			
Conventionals						
Chromium (VI)	0.144	1.00	ND			

QUALITY CONTROL REPORT

QC Batch No: 100108-2

Analytes	LCS % REC	LCS/LCSD % Limit							
Conventionals									
Chromium (VI)	101	90-110							



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Oakland, CA 94610-

Site

9037 Melrose Ave.
West Hollywood, CA 90069

Telephone: (510)628-3228

Attn: Warren Chamberlain

Page: 5

Project ID: 4088.08.7937.02
Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 300, Anions by Ion Chromatography

QC Batch No: 100108-2

Our Lab I.D.			225768			
Client Sample I.D.			EW-1			
Date Sampled			10/01/2008			
Date Prepared			10/01/2008			
Preparation Method						
Date Analyzed			10/01/2008			
Matrix			Water			
Units			mg/L			
Dilution Factor			1			
Analytes	MDL	PQL	Results			
Conventionals						
Chloride	0.241	1.00	128			
Nitrate as N	0.0070	0.100	0.280			
Nitrite as N	0.0052	0.0500	0.0900			
Sulfate	0.0700	1.00	181			

QUALITY CONTROL REPORT

QC Batch No: 100108-2

Analytes	LCS % REC	LCS/LCSD % Limit							
Conventionals									
Chloride	103	80-120							
Nitrate as N	102	80-120							
Nitrite as N	108	80-120							
Sulfate	103	80-120							



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 Oakland, CA 94610-

Site

9037 Melrose Ave.
 West Hollywood, CA 90069

Telephone: (510)628-3228

Attn: Warren Chamberlain

Page: 6

Project ID: 4088.08.7937.02
 Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 314.0, Perchlorate by Ion Chromatography

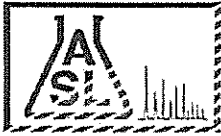
QC Batch No: 100608-1

Our Lab I.D.		225768			
Client Sample I.D.		EW-1			
Date Sampled		10/01/2008			
Date Prepared		10/06/2008			
Preparation Method					
Date Analyzed		10/06/2008			
Matrix		Water			
Units		ug/L			
Dilution Factor		1			
Analytes	MDL	PQL	Results		
Conventionals					
Perchlorate	0.450	2.00	ND		

QUALITY CONTROL REPORT

QC Batch No: 100608-1

Analytes	LCS % REC	LCS/LCSD % Limit							
Conventionals									
Perchlorate	102	85-115							



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Mactec
 600 Grand Ave. Suite # 300
 Oakland, CA 94610-

Site

9037 Melrose Ave.
 West Hollywood, CA 90069

Telephone: (510)628-3228

Attn: Warren Chamberlain

Page: 7

Project ID: 4088.08.7937.02

Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 6010B/7470A, CCR Title 22 Metals (TTLC)

QC Batch No: 100308-1

Our Lab I.D.	225768				
Client Sample I.D.	EW-1				
Date Sampled	10/01/2008				
Date Prepared	10/03/2008				
Preparation Method					
Date Analyzed	10/03/2008				
Matrix	Water				
Units	ug/L				
Dilution Factor	1				
Analytes	MDL	PQL	Results		
AA Metals					
Mercury	0.100	0.500	ND		
ICP Metals					
Boron	9.00	50.0	179		
Antimony	2.00	10.0	ND		
Arsenic	2.00	10.0	ND		
Beryllium	0.500	10.0	ND		
Cadmium	0.500	10.0	ND		
Chromium	1.00	10.0	ND		
Nickel	1.00	10.0	ND		
Thallium	1.00	10.0	ND		
Zinc	1.00	10.0	ND		

Comment(s):

note

QUALITY CONTROL REPORT

QC Batch No: 100308-1

Analytes	LCS % REC	LCS/LCSD % Limit						
AA Metals								
Mercury	95	80-120						
ICP Metals								
Boron	96	80-120						
Antimony	98	80-120						
Arsenic	99	80-120						
Beryllium	106	80-120						
Cadmium	97	80-120						
Chromium	100	80-120						
Nickel	101	80-120						



AMERICAN SCIENTIFIC LABORATORIES, LLC

Environmental Testing Services

2520 N. San Fernando Rd., Los Angeles, CA 90065 Tel: (323) 223-9700 Fax: (323) 223-9500

ANALYTICAL RESULTS

Ordered By

Mactec
600 Grand Ave. Suite # 300
Oakland, CA 94610-

Site

9037 Melrose Ave.
West Hollywood, CA 90069

Telephone: (510)628-3228

Attn: Warren Chamberlain

Page: 9

Project ID: 4088.08.7937.02

Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 6010B/7470A, Priority Pollutant Metals

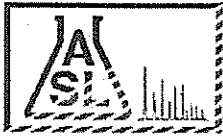
QC Batch No: 100308-1

Our Lab I.D.		225766	225767			
Client Sample I.D.		OW-1	OW-3			
Date Sampled		10/01/2008	10/01/2008			
Date Prepared		10/03/2008	10/03/2008			
Preparation Method						
Date Analyzed		10/03/2008	10/03/2008			
Matrix		Water	Water			
Units		mg/L	mg/L			
Dilution Factor		1	1			
Analytes	MDL	PQL	Results	Results		
AA Metals						
Mercury	0.0001	0.0005	ND	ND		
ICP Metals						
Antimony	0.0020	0.0100	ND	ND		
Arsenic	0.0020	0.0100	ND	ND		
Beryllium	0.0005	0.0050	ND	ND		
Cadmium	0.0005	0.0050	ND	ND		
Chromium	0.0010	0.0100	ND	ND		
Copper	0.0010	0.0100	ND	ND		
Lead	0.0020	0.0050	ND	ND		
Nickel	0.0010	0.0100	ND	ND		
Selenium	0.0040	0.0100	ND	ND		
Silver	0.0030	0.0100	ND	ND		
Thallium	0.0010	0.0100	ND	ND		
Zinc	0.0070	0.0100	ND	ND		

QUALITY CONTROL REPORT

QC Batch No: 100308-1

Analytes	LCS % REC	LCS/LCSD % Limit						
AA Metals								
Mercury	95	80-120						
ICP Metals								
Antimony	98	80-120						
Arsenic	98	80-120						
Beryllium	106	80-120						
Cadmium	97	80-120						
Chromium	100	80-120						
Copper	102	80-120						



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Environmental Testing Services

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 Oakland, CA 94610-

Site

9037 Melrose Ave.
 West Hollywood, CA 90069

Telephone: (510)628-3228

Attn: Warren Chamberlain

Page: 11

Project ID: 4088.08.7937.02

Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 6020, Metals by ICP/MS

QC Batch No: 100308-1

Our Lab I.D.	225768		
Client Sample I.D.	EW-1		
Date Sampled	10/01/2008		
Date Prepared	10/03/2008		
Preparation Method			
Date Analyzed	10/03/2008		
Matrix	Water		
Units	ug/L		
Dilution Factor	1		
Analytes	MDL	PQL	Results
ICP Metals			
Copper	0.0610	2.00	0.540J
Lead	0.110	1.00	ND
Selenium	0.230	2.00	ND
Silver	0.190	0.500	ND

QUALITY CONTROL REPORT

QC Batch No: 100308-1

Analytes	LCS % REC	LCS/LCSD % Limit							
ICP Metals									
Copper	100	85-115							
Lead	103	85-115							
Selenium	104	85-115							
Silver	100	85-115							



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ANALYTICAL RESULTS

Ordered By

Mactec
 600 Grand Ave. Suite #300
 Oakland, CA 94610-

Site

9037 Melrose Ave.
 West Hollywood, CA 90069

Telephone: (510)628-3228

Attn: Warren Chamberlain

Page: 12

Project ID: 4088.08.7937.02

Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 8015B, TPH DROs and OROs (Diesel and Oil Range Organics)

QC Batch No: 100208-2D

Our Lab I.D.		225768				
Client Sample I.D.		EW-1				
Date Sampled		10/01/2008				
Date Prepared		10/02/2008				
Preparation Method						
Date Analyzed		10/03/2008				
Matrix		Water				
Units		mg/L				
Dilution Factor		1				
Analytes	MDL	PQL	Results			
TPH DROs (C10 to C28)	0.0500	0.500	ND			
TPH OROs (C28+)	0.170	0.500	ND			

Our Lab I.D.		225768				
Surrogates	% Rec.Limit	% Rec.				
Surrogate Percent Recovery						
Chlorobenzene	70-120	79				

QUALITY CONTROL REPORT

QC Batch No: 100208-2D

Analytes	MS % REC	MS DUP % REC	RPD %	MS/MSD % Limit	MS RPD % Limit				
Diesel	94	100	6.2	75-120	<20				



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Project ID: 4088.08.7937.02

Project Name: Melrose Traingle

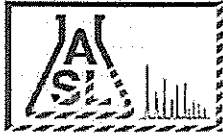
ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 8081A, Organochlorine Pesticides

QC Batch No: 100208-1

Our Lab I.D.		225768			
Client Sample I.D.		EW-1			
Date Sampled		10/01/2008			
Date Prepared		10/02/2008			
Preparation Method					
Date Analyzed		10/02/2008			
Matrix		Water			
Units		ug/L			
Dilution Factor		1			
Analytes	MDL	PQL	Results		
Aldrin	0.0070	0.0400	ND		
alpha-Hexachlorocyclohexane (Alpha-BHC)	0.0090	0.120	ND		
Beta-Hexachlorocyclohexane (Beta-BHC)	0.0050	0.110	ND		
alpha-Chlordane	0.0090	0.400	ND		
Gamma-Chlordane	0.0170	0.400	ND		
4,4'-DDD (DDD)	0.0080	0.100	ND		
4,4'-DDE (DDE)	0.0070	0.0900	ND		
4,4'-DDT (DDT)	0.0060	0.0400	ND		
delta-Hexachlorocyclohexane (Delta-BHC)	0.0050	0.110	ND		
dieldrin	0.0090	0.0500	ND		
Endosulfan 1	0.0100	0.0600	ND		
Endosulfan 11	0.0090	0.0900	ND		
Endosulfan sulfate	0.0100	0.0700	ND		
Endrin	0.0090	0.0800	ND		
Endrin aldehyde	0.0150	0.0900	ND		
Endrin ketone	0.0060	0.0700	ND		
gamma-Hexachlorocyclohexane (Gamma-BHC, Lindane)	0.0110	0.0600	ND		
Heptachlor	0.0020	0.0300	ND		
Heptachlor epoxide	0.0050	0.0500	ND		
Methoxychlor	0.0050	10.0	ND		
Toxaphene	2.00	10.0	ND		

Our Lab I.D.		225768			
Surrogates	% Rec.Limit	% Rec.			
Surrogate Percent Recovery					
Decachlorobiphenyl		111			



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ANALYTICAL RESULTS

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Project ID: 4088.08.7937.02
Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 8081A, Organochlorine Pesticides
QUALITY CONTROL REPORT

QC Batch No: 100208-1

Analytes	LCS % REC	LCS DUP % REC	LCS RPD % REC	LCS/LCSD % Limit	LCS RPD % Limit					
Aldrin	82	98	17.8	42-122						
4,4'-DDT (DDT)	28	30	6.9	25-160						
dieldrin	96	98	2.1	36-146						
Endrin	98	103	5.0	30-147						
gamma-Hexachlorocyclohexane (Gamma-BHC, Lindane)	81	86	6.0	32-127						
Heptachlor	69	84	19.6	34-111						



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Page: 15

Project ID: 4088.08.7937.02

Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 8082, Polychlorinated Biphenyls(PCBs) by Gas Chromatography

QC Batch No: 100208-1

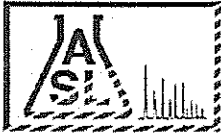
Our Lab I.D.	225768		
Client Sample I.D.	EW-1		
Date Sampled	10/01/2008		
Date Prepared	10/02/2008		
Preparation Method			
Date Analyzed	10/02/2008		
Matrix	Water		
Units	ug/L		
Dilution Factor	1		
Analytes	MDL	PQL	Results
Aroclor-1016 (PCB-1016)	0.279	0.650	ND
Aroclor-1221 (PCB-1221)	0.558	1.00	ND
Aroclor-1232 (PCB-1232)	0.225	0.650	ND
Aroclor-1242 (PCB-1242)	0.225	0.650	ND
Aroclor-1248 (PCB-1248)	0.225	0.650	ND
Aroclor-1254 (PCB-1254)	0.225	0.650	ND
Aroclor-1260 (PCB-1260)	0.225	0.650	ND

Our Lab I.D.	225768		
Surrogates	% Rec.Limit	% Rec.	
Surrogate Percent Recovery			
Decachlorobiphenyl	43-169	111	

QUALITY CONTROL REPORT

QC Batch No: 100208-1

Analytes	LCS % REC	LCS DUP % REC	LCS RPD % REC	LCS/LCSD % Limit	LCS RPD % Limit
Aroclor-1260 (PCB-1260)	115	116	<1	39-150	



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ANALYTICAL RESULTS

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Page: 16

Project ID: 4088.08.7937.02

Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 8260B, Volatile Organic Compounds

QC Batch No: 100208-2C

Our Lab I.D.		225766	225767			
Client Sample I.D.		OW-1	OW-3			
Date Sampled		10/01/2008	10/01/2008			
Date Prepared		10/03/2008	10/03/2008			
Preparation Method						
Date Analyzed		10/03/2008	10/03/2008			
Matrix		Water	Water			
Units		ug/L	ug/L			
Dilution Factor		1	1			
Analytes	MDL	PQL	Results	Results		
Acetone	2.52	5.00	ND	ND		
Benzene	0.0970	1.00	ND	ND		
Bromobenzene (Phenyl bromide)	0.291	1.00	ND	ND		
Bromochloromethane (Chlorobromomethane)	0.169	1.00	ND	ND		
Bromodichloromethane (Dichlorobromomethane)	0.169	1.00	ND	ND		
Bromoform (Tribromomethane)	0.284	5.00	ND	ND		
Bromomethane (Methyl bromide)	0.174	3.00	ND	ND		
2-Butanone (MEK; Methyl ethyl ketone)	5.00	5.00	ND	ND		
n-Butylbenzene	0.363	1.00	ND	ND		
sec-Butylbenzene	0.338	1.00	ND	ND		
tert-Butylbenzene	0.235	1.00	ND	ND		
Carbon disulfide	0.463	1.00	ND	ND		
Carbon tetrachloride (Tetrachloromethane)	0.144	1.00	ND	ND		
Chlorobenzene	0.176	1.00	ND	ND		
Chloroethane	0.328	3.00	ND	ND		
2-Chloroethyl vinyl ether	0.665	5.00	ND	ND		
Chloroform (Trichloromethane)	0.247	1.00	ND	ND		
Chloromethane (Methyl chloride)	0.174	3.00	ND	ND		
4-Chlorotoluene (p-Chlorotoluene)	0.311	1.00	ND	ND		
2-Chlorotoluene (o-Chlorotoluene)	0.147	1.00	ND	ND		
1,2-Dibromo-3-chloropropane (DBCP)	0.333	5.00	ND	ND		
Dibromochloromethane	0.300	1.00	ND	ND		
1,2-Dibromoethane (EDB, Ethylene dibromide)	0.226	1.00	ND	ND		
Dibromomethane	0.316	1.00	ND	ND		
1,2-Dichlorobenzene (o-Dichlorobenzene)	0.358	1.00	ND	ND		
1,3-Dichlorobenzene (m-Dichlorobenzene)	0.333	1.00	ND	ND		



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ANALYTICAL RESULTS

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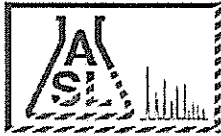
Project ID: 4088.08.7937.02
 Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 8260B, Volatile Organic Compounds

QC Batch No: 100208-2C

Our Lab I.D.		225766	225767		
Client Sample I.D.		OW-1	OW-3		
Date Sampled		10/01/2008	10/01/2008		
Date Prepared		10/03/2008	10/03/2008		
Preparation Method					
Date Analyzed		10/03/2008	10/03/2008		
Matrix		Water	Water		
Units		ug/L	ug/L		
Dilution Factor		1	1		
Analytes	MDL	PQL	Results	Results	
1,4-Dichlorobenzene (p-Dichlorobenzene)	0.384	1.00	ND	ND	
Dichlorodifluoromethane	0.244	3.00	ND	ND	
1,1-Dichloroethane	0.372	1.00	ND	ND	
1,2-Dichloroethane	0.182	1.00	ND	ND	
1,1-Dichloroethene (1,1-Dichloroethylene)	0.355	1.00	ND	ND	
cis-1,2-Dichloroethene	0.279	1.00	ND	ND	
trans-1,2-Dichloroethene	0.176	1.00	ND	ND	
1,2-Dichloropropane	0.359	1.00	ND	ND	
1,3-Dichloropropane	0.205	1.00	ND	ND	
2,2-Dichloropropane	0.341	1.00	ND	ND	
1,1-Dichloropropene	0.210	1.00	ND	ND	
cis-1,3-Dichloropropene	0.122	1.00	ND	ND	
trans-1,3-Dichloropropene	0.100	1.00	ND	ND	
Ethylbenzene	0.209	1.00	ND	ND	
Hexachlorobutadiene (1,3-Hexachlorobutadiene)	0.413	3.00	ND	ND	
2-Hexanone	0.944	5.00	ND	ND	
Isopropylbenzene	0.291	1.00	ND	ND	
p-Isopropyltoluene (4-Isopropyltoluene)	0.468	1.00	ND	ND	
MTBE	0.240	2.00	ND	ND	
4-Methyl-2-pentanone (MIBK, Methyl isobutyl ketone)	1.71	5.00	ND	ND	
Methylene chloride (Dichloromethane, DCM)	1.00	5.00	ND	ND	
Naphthalene	0.375	1.00	ND	ND	
n-Propylbenzene	0.254	1.00	ND	ND	
Styrene	0.122	1.00	ND	ND	
1,1,1,2-Tetrachloroethane	0.141	1.00	ND	ND	
1,1,2,2-Tetrachloroethane	0.579	1.00	ND	ND	
Tetrachloroethene (Tetrachloroethylene)	0.421	1.00	ND	ND	
Toluene (Methyl benzene)	0.282	1.00	ND	ND	
1,2,3-Trichlorobenzene	0.219	1.00	ND	ND	
1,2,4-Trichlorobenzene	0.451	1.00	ND	ND	
1,1,1-Trichloroethane	0.150	1.00	ND	ND	
1,1,2-Trichloroethane	0.233	1.00	ND	ND	



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Environmental Testing Services

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ANALYTICAL RESULTS

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Project ID: 4088.08.7937.02

Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 8260B, Volatile Organic Compounds

QC Batch No: 100208-2C

Our Lab I.D.		225766	225767			
Client Sample I.D.		OW-1	OW-3			
Date Sampled		10/01/2008	10/01/2008			
Date Prepared		10/03/2008	10/03/2008			
Preparation Method						
Date Analyzed		10/03/2008	10/03/2008			
Matrix		Water	Water			
Units		ug/L	ug/L			
Dilution Factor		1	1			
Analytes	MDL	PQL	Results	Results		
Trichloroethene (TCE)	0.117	1.00	ND	ND		
Trichlorofluoromethane	0.294	1.00	ND	ND		
1,2,3-Trichloropropane	0.303	1.00	ND	ND		
1,2,4-Trimethylbenzene	0.451	1.00	ND	ND		
1,3,5-Trimethylbenzene	0.219	1.00	ND	ND		
Vinyl acetate	1.62	5.00	ND	ND		
Vinyl chloride (Chloroethene)	0.331	3.00	ND	ND		
o-Xylene	0.262	1.00	ND	ND		
m- & p-Xylenes	0.476	2.00	ND	ND		

Our Lab I.D.		225766	225767			
Surrogates	% Rec.Limit	% Rec.	% Rec.			
Surrogate Percent Recovery						
Bromofluorobenzene	70-120	101	98			
Dibromofluoromethane	70-120	112	118			
Toluene-d8	70-120	99	100			

QUALITY CONTROL REPORT

QC Batch No: 100208-2C

Analytes	MS % REC	MS DUP % REC	RPD %	MS/MSD % Limit	MS RPD % Limit				
Benzene	87	92	5.6	75-120	15				
Chlorobenzene	96	100	4.1	75-120	15				
1,1-Dichloroethene (1,1-Dichloroethylene)	106	99	6.8	75-120	15				
MTBE	95	91	4.3	75-120	15				
Toluene (Methyl benzene)	96	101	5.1	75-120	15				
Trichloroethene (TCE)	94	100	6.2	75-120	15				



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ANALYTICAL RESULTS

Ordered By

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Site

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Telephone: (510)628-3228

Attn: Warren Chamberlain

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Project ID: 4088.08.7937.02

Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 8260B, Volatile Organic Compounds + Oxygenates

QC Batch No: 100308-1B

Our Lab I.D.		225768			
Client Sample I.D.		EW-1			
Date Sampled		10/01/2008			
Date Prepared		10/03/2008			
Preparation Method					
Date Analyzed		10/03/2008			
Matrix		Water			
Units		ug/L			
Dilution Factor		1			
Analytes	MDL	PQL	Results		
Acrolein (2-Propenal)	5.00	50.0	ND		
Acrylonitrile (2-Propenenitrile)	2.00	50.0	ND		
Acetone	2.52	5.00	ND		
Benzene	0.0970	0.500	ND		
Bromobenzene (Phenyl bromide)	0.291	1.00	ND		
Bromochloromethane (Chlorobromomethane)	0.169	1.00	ND		
Bromodichloromethane (Dichlorobromomethane)	0.169	0.500	ND		
Bromoform (Tribromomethane)	0.284	0.500	ND		
Bromomethane (Methyl bromide)	0.174	2.00	ND		
2-Butanone (MEK, Methyl ethyl ketone)	5.00	5.00	ND		
n-Butylbenzene	0.363	1.00	ND		
sec-Butylbenzene	0.338	1.00	ND		
tert-Butylbenzene	0.235	1.00	ND		
Carbon disulfide	0.463	1.00	ND		
Carbon tetrachloride (Tetrachloromethane)	0.144	0.500	ND		
Chlorobenzene	0.176	2.00	ND		
Chloroethane	0.328	2.00	ND		
2-Chloroethyl vinyl ether	0.665	1.00	ND		
Chloroform (Trichloromethane)	0.247	2.00	ND		
Chloromethane (Methyl chloride)	0.174	3.00	ND		
4-Chlorotoluene (p-Chlorotoluene)	0.311	1.00	ND		
DIPE	0.530	2.00	ND		
2-Chlorotoluene (o-Chlorotoluene)	0.147	1.00	ND		
1,2-Dibromo-3-chloropropane (DBCP)	0.333	5.00	ND		
Dibromochloromethane	0.300	0.500	ND		
1,2-Dibromoethane (EDB, Ethylene dibromide)	0.226	1.00	ND		



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ANALYTICAL RESULTS

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Project ID: 4088.08.7937.02
 Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 8260B, Volatile Organic Compounds + Oxygenates

QC Batch No: 100308-1B

Our Lab I.D.	225768		
Client Sample I.D.	EW-1		
Date Sampled	10/01/2008		
Date Prepared	10/03/2008		
Preparation Method			
Date Analyzed	10/03/2008		
Matrix	Water		
Units	ug/L		
Dilution Factor	1		
Analytes	MDL	PQL	Results
Dibromomethane	0.316	1.00	ND
1,2-Dichlorobenzene (o-Dichlorobenzene)	0.358	0.500	ND
1,3-Dichlorobenzene (m-Dichlorobenzene)	0.333	2.00	ND
1,4-Dichlorobenzene (p-Dichlorobenzene)	0.384	0.500	ND
Dichlorodifluoromethane	0.244	3.00	ND
1,1-Dichloroethane	0.372	1.00	ND
1,2-Dichloroethane	0.182	0.500	ND
1,1-Dichloroethene (1,1-Dichloroethylene)	0.355	1.00	ND
cis-1,2-Dichloroethene	0.279	1.00	ND
trans-1,2-Dichloroethene	0.176	1.00	ND
1,2-Dichloropropane	0.359	0.500	ND
1,3-Dichloropropane	0.205	1.00	ND
2,2-Dichloropropane	0.341	1.00	ND
1,1-Dichloropropene	0.210	1.00	ND
cis-1,3-Dichloropropene	0.122	0.500	ND
ETBE	0.460	2.00	ND
trans-1,3-Dichloropropene	0.100	1.00	ND
Ethylbenzene	0.209	1.00	ND
Hexachlorobutadiene (1,3-Hexachlorobutadiene)	0.413	3.00	ND
2-Hexanone	0.944	5.00	ND
Isopropylbenzene	0.291	1.00	ND
p-Isopropyltoluene (4-Isopropyltoluene)	0.468	1.00	ND
MTBE	0.240	2.00	ND
4-Methyl-2-pentanone (MIBK, Methyl isobutyl ketone)	1.71	5.00	ND
Methylene chloride (Dichloromethane, DCM)	0.500	0.500	ND
Naphthalene	0.375	1.00	ND
n-Propylbenzene	0.254	1.00	ND
TAME	0.440	2.00	ND
Styrene	0.122	1.00	ND
TBA	4.75	10.0	ND
1,1,1,2-Tetrachloroethane	0.141	1.00	ND
1,1,2,2-Tetrachloroethane	0.500	0.500	ND



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ANALYTICAL RESULTS

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Project ID: 4088.08.7937.02
 Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 8260B, Volatile Organic Compounds + Oxygenates

QC Batch No: 100308-1B

Our Lab I.D.	225768		
Client Sample I.D.	EW-1		
Date Sampled	10/01/2008		
Date Prepared	10/03/2008		
Preparation Method			
Date Analyzed	10/03/2008		
Matrix	Water		
Units	ug/L		
Dilution Factor	1		
Analytes	MDL	PQL	Results
Tetrachloroethene (Tetrachloroethylene)	0.421	0.500	ND
Toluene (Methyl benzene)	0.282	2.00	ND
1,2,3-Trichlorobenzene	0.219	1.00	ND
1,2,4-Trichlorobenzene	0.451	1.00	ND
1,1,1-Trichloroethane	0.150	2.00	ND
1,1,2-Trichloroethane	0.233	0.500	ND
Trichloroethene (TCE)	0.117	0.500	ND
Trichlorofluoromethane	0.294	1.00	ND
1,2,3-Trichloropropane	0.303	1.00	ND
1,2,4-Trimethylbenzene	0.451	1.00	ND
1,3,5-Trimethylbenzene	0.219	1.00	ND
Vinyl acetate	1.62	5.00	ND
Vinyl chloride (Chloroethene)	0.331	0.500	ND
o-Xylene	0.262	1.00	ND
m- & p-Xylenes	0.476	2.00	ND

Our Lab I.D.	225768		
Surrogates	% Rec.Limit	% Rec.	
Surrogate Percent Recovery			
Bromofluorobenzene	70-120	104	
Dibromofluoromethane	70-120	78	
Toluene-d8	70-120	94	

QUALITY CONTROL REPORT

QC Batch No: 100308-1B

Analytes	MS % REC	MS DUP % REC	RPD %	MS/MSD % Limit	MS RPD % Limit
Benzene	101	100	<1	75-120	15
Chlorobenzene	88	90	2.2	75-120	15
1,1-Dichloroethene (1,1-Dichloroethylene)	112	114	1.8	75-120	15
MTBE	98	96	2.1	75-120	15
Toluene (Methyl benzene)	93	92	1.1	75-120	15
Trichloroethene (TCE)	101	100	<1	75-120	15



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ANALYTICAL RESULTS

Ordered By

Mactec
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Site

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 West Hollywood, CA 90069

Telephone: (510)628-3228

Attn: Warren Chamberlain

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Project ID: 4088.08.7937.02

Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 8260B, TPH GROs(Gasoline Range Organics)

QC Batch No: 100308-1B

Our Lab I.D.	225768		
Client Sample I.D.	EW-1		
Date Sampled	10/01/2008		
Date Prepared	10/03/2008		
Preparation Method			
Date Analyzed	10/03/2008		
Matrix	Water		
Units	ug/L		
Dilution Factor	1		
Analytes	MDL	PQL	Results
TPH GROs (C6 to C10)	50.0	50.0	ND

Our Lab I.D.	225768		
Surrogates	% Rec.Limit	% Rec.	
Surrogate Percent Recovery			
Bromofluorobenzene	70-120	104	
Dibromofluoromethane	70-120	78	
Toluene-d8	70-120	94	

QUALITY CONTROL REPORT

QC Batch No: 100308-1B

Analytes	MS % REC	MS DUP % REC	RPD %	MS/MSD % Limit	MS RPD % Limit
Benzene	101	100	<1	75-120	15
Chlorobenzene	88	90	2.2	75-120	15
1,1-Dichloroethene (1,1-Dichloroethylene)	112	114	1.8	75-120	15
Toluene (Methyl benzene)	93	92	1.1	75-120	15
Trichloroethene (TCE)	101	100	<1	75-120	15



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Project ID: 4088.08.7937.02
 Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 8270C, 1,4-Dioxane

QC Batch No: 100308-1

Our Lab I.D.	225768		
Client Sample I.D.	EW-1		
Date Sampled	10/01/2008		
Date Prepared	10/02/2008		
Preparation Method			
Date Analyzed	10/03/2008		
Matrix	Water		
Units	ug/L		
Dilution Factor	1		
Analytes	MDL	PQL	Results
1,4-Dioxane	2.00	10.0	ND

Our Lab I.D.	225768	
Surrogates	% Rec.Limit	% Rec.
Surrogate Percent Recovery		
2-Fluorophenol	21-105	41
Phenol-d6	10-107	25
2,4,6-Tribromophenol	10-123	59
Nitrobenzene-d5	35-114	53
2-Fluorobiphenyl	18-116	59
Terphenyl-d14	33-141	77

QUALITY CONTROL REPORT

QC Batch No: 100308-1

Analytes	LCS % REC	LCS DUP % REC	LCS RPD % REC	LCS/LCSD % Limit	LCS RPD % Limit
Acenaphthene	53	46	14.1	43-118	<30
4-Chloro-3-methylphenol (p-Chloro-m-cresol)	49	44	10.8	23-117	<30
2-Chlorophenol (o-Chlorophenol)	40	35	13.3	27-113	<30
1,4-Dichlorobenzene	41	35	15.8	36-105	<30
2,4-Dinitrotoluene	53	51	3.8	24-120	<30
N-Nitroso-Di-n-propylamine	68	56	19.4	41-116	<30
4-Nitrophenol	20	23	14.0	10-133	<30
Pentachlorophenol	28	23	19.6	9-118	<30
Phenol	19	20	5.1	12-110	<30
Pyrene	61	70	13.7	26-127	<30
1,2,4-Trichlorobenzene	50	48	4.1	39-98	<30



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Project ID: 4088.08.7937.02

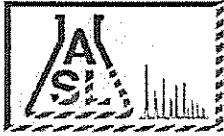
Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 8270C, Semivolatile Organics

QC Batch No: 100308-1

Our Lab I.D.				225768			
Client Sample I.D.				EW-1			
Date Sampled				10/01/2008			
Date Prepared				10/02/2008			
Preparation Method							
Date Analyzed				10/03/2008			
Matrix				Water			
Units				ug/L			
Dilution Factor				1			
Analytes	MDL	PQL	Results				
Acenaphthene	1.86	10.0	ND				
Acenaphthylene	1.83	10.0	ND				
Anthracene	1.92	10.0	ND				
Benz(a)anthracene (Benzo(a)anthracene)	1.41	10.0	ND				
Benzo(a)pyrene	1.47	10.0	ND				
Benzo(b)fluoranthene	1.38	10.0	ND				
Benzo(ghi)perylene	1.98	10.0	ND				
Benzo(k)fluoranthene	1.44	10.0	ND				
Benzoic acid	1.29	10.0	ND				
Benzidine	5.00	10.0	ND				
Benzyl alcohol	1.98	10.0	ND				
Bis(2-chloroethoxy)methane	1.77	10.0	ND				
Bis(2-chloroethyl)ether	1.29	10.0	ND				
Bis(2-chloroisopropyl) ether	1.47	10.0	ND				
Bis(2-ethylhexyl) phthalate	0.780	10.0	ND				
4-Bromophenyl phenyl ether	2.19	10.0	ND				
Butyl benzyl phthalate (Benzyl butyl phthalate)	2.73	10.0	ND				
4-Chloro-3-methylphenol (p-Chloro-m-cresol)	1.00	1.00	ND				
4-Chloroaniline	1.74	10.0	ND				
2-Chloronaphthalene	1.74	10.0	ND				
2-Chlorophenol (o-Chlorophenol)	0.950	1.00	ND				
4-Chlorophenyl phenyl ether	1.98	10.0	ND				
Chrysene	1.41	10.0	ND				
Di-n-butyl phthalate	1.14	10.0	ND				
Di-n-octyl phthalate (Dioctyl ester)	1.80	10.0	ND				
Dibenz(a,h)anthracene	1.92	10.0	ND				
Dibenzofuran	1.95	10.0	ND				



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ANALYTICAL RESULTS

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Project ID: 4088.08.7937.02
 Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 8270C, Semivolatile Organics

QC Batch No: 100308-1

Our Lab I.D.	225768		
Client Sample I.D.	EW-1		
Date Sampled	10/01/2008		
Date Prepared	10/02/2008		
Preparation Method			
Date Analyzed	10/03/2008		
Matrix	Water		
Units	ug/L		
Dilution Factor	1		
Analytes	MDL	PQL	Results
1,3-Dichlorobenzene (m-Dichlorobenzene)	1.32	10.0	ND
1,2-Dichlorobenzene (o-Dichlorobenzene)	1.38	10.0	ND
1,4-Dichlorobenzene	1.38	10.0	ND
3,3'-Dichlorobenzidine	2.40	20.0	ND
2,4-Dichlorophenol	1.95	1.00	ND
Diethyl phthalate (Diethyl ester)	1.74	10.0	ND
2,4-Dimethylphenol	1.74	1.00	ND
Dimethyl phthalate (Dimethyl ester)	3.75	10.0	ND
2,4-Dinitrophenol	2.34	1.00	ND
2,4-Dinitrotoluene	0.880	10.0	ND
1,4-Dioxane	3.00	10.0	ND
2,6-Dinitrotoluene (2,6-DNT)	1.77	10.0	ND
1,2-Diphenylhydrazine	1.00	10.0	ND
Fluoranthene	1.59	10.0	ND
Fluorene	1.92	10.0	ND
Hexachlorobenzene	2.01	10.0	ND
Hexachlorobutadiene (1,3-Hexachlorobutadiene)	1.05	20.0	ND
Hexachlorocyclopentadiene	0.720	10.0	ND
Hexachloroethane	1.11	10.0	ND
Indeno(1,2,3-cd)pyrene	1.92	10.0	ND
Isophorone	1.74	10.0	ND
2-methyl-4,6-Dinitrophenol	1.74	1.00	ND
2-Methylnaphthalene	1.12	10.0	ND
2-Methylphenol (o-Cresol, 2-Cresol)	0.840	1.00	ND
4-Methylphenol (p-Cresol, 4-Cresol)	0.560	1.00	ND
N-Nitroso-Di-n-propylamine	1.74	10.0	ND
N-Nitrosodiphenylamine	1.65	10.0	ND
Naphthalene	1.74	10.0	ND
2-Nitroaniline	1.65	10.0	ND
3-Nitroaniline	1.71	10.0	ND
4-Nitroaniline	1.80	10.0	ND
Nitrobenzene (NB)	1.53	10.0	ND
2-Nitrophenol (o-Nitrophenol)	4.44	1.00	ND
N-Nitrosodimethylamine (NDMA)	5.00	10.0	ND



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ANALYTICAL RESULTS

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Project ID: 4088.08.7937.02
 Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: 8270C, Semivolatile Organics

QC Batch No: 100308-1

Our Lab I.D.	225768		
Client Sample I.D.	EW-1		
Date Sampled	10/01/2008		
Date Prepared	10/02/2008		
Preparation Method			
Date Analyzed	10/03/2008		
Matrix	Water		
Units	ug/L		
Dilution Factor	1		
Analytes	MDL	PQL	Results
4-Nitrophenol	2.04	1.00	ND
Pentachlorophenol	1.00	1.00	ND
Phenanthrene	2.13	10.0	ND
Phenol	0.840	1.00	ND
Pyrene	1.83	10.0	ND
1,2,4-Trichlorobenzene	1.62	10.0	ND
2,4,5-Trichlorophenol	0.870	1.00	ND
2,4,6-Trichlorophenol	0.990	1.00	ND

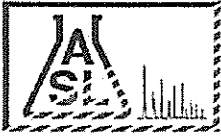
Our Lab I.D.	225768		
Surrogates	% Rec.Limit	% Rec.	
Surrogate Percent Recovery			
2-Fluorophenol	21-105	41	
Phenol-d6	10-107	25	
2,4,6-Tribromophenol	10-123	59	
Nitrobenzene-d5	35-114	53	
2-Fluorobiphenyl	43-116	59	
Terphenyl-d14	33-141	77	

QUALITY CONTROL REPORT

QC Batch No: 100308-1

Analytes	LCS % REC	LCS DUP % REC	LCS RPD % REC	LCS/LCSD % Limit	LCS RPD % Limit
Acenaphthene	53	46	14.1	46-118	<30
4-Chloro-3-methylphenol (p-Chloro-m-cresol)	49	44	10.8	23-117	<30
2-Chlorophenol (o-Chlorophenol)	40	35	13.3	27-113	<30
1,4-Dichlorobenzene	41	35	15.8	36-105	<30
2,4-Dinitrotoluene	53	51	3.8	24-120	<30
N-Nitroso-Di-n-propylamine	68	56	19.4	41-116	<30
4-Nitrophenol	20	23	14.0	10-133	<30
Pentachlorophenol	28	23	19.6	9-118	<30
Phenol	19	20	5.1	12-110	<30
Pyrene	61	70	13.7	26-127	<30
1,2,4-Trichlorobenzene	50	48	4.1	39-98	<30

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ANALYTICAL RESULTS

Ordered By

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Site

9037 Melrose Ave.
 West Hollywood, CA 90069

Telephone: (510)628-3228

Attn: Warren Chamberlain

Page: 28

Project ID: 4088.08.7937.02

Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: SM2340-C, Hardness (EDTA Titrimetric Method)

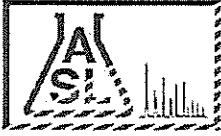
QC Batch No: 100108-2

Our Lab I.D.		225768			
Client Sample I.D.		EW-1			
Date Sampled		10/01/2008			
Date Prepared		10/02/2008			
Preparation Method					
Date Analyzed		10/02/2008			
Matrix		Water			
Units		mg/L			
Dilution Factor		1			
Analytes	MDL	PQL	Results		
Conventionals					
Hardness (Ca,Mg) as CaCO3	5.00	10.0	530		

QUALITY CONTROL REPORT

QC Batch No: 100108-2

Analytes	LCS % REC	LCS/LCSD % Limit							
Conventionals									
Hardness (Ca,Mg) as CaCO3	100	80-120							



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Project ID: 4088.08.7937.02

Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: SM2540-C, Total Dissolved Solids (TDS)

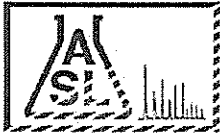
QC Batch No: 100108-2

Our Lab I.D.		225768				
Client Sample I.D.		EW-1				
Date Sampled		10/01/2008				
Date Prepared		10/02/2008				
Preparation Method						
Date Analyzed		10/02/2008				
Matrix		Water				
Units		mg/L				
Dilution Factor		1				
Analytes	MDL	PQL	Results			
Conventionals						
Solids, Total Dissolved (TDS)	5.00	10.0	880			

QUALITY CONTROL REPORT

QC Batch No: 100108-2

Analytes	LCS % REC	LCS/LCSD % Limit							
Conventionals									
Solids, Total Dissolved (TDS)	96	80-120							



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Project ID: 4088.08.7937.02
 Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: SM2540-D, Total Suspended Solids (TSS)

QC Batch No: 100208-1

Our Lab I.D.	225768		
Client Sample I.D.	EW-1		
Date Sampled	10/01/2008		
Date Prepared	10/02/2008		
Preparation Method			
Date Analyzed	10/02/2008		
Matrix	Water		
Units	mg/L		
Dilution Factor	1		
Analytes	MDL	PQL	Results
Conventionals			
Solids, Total Suspended (TSS)	5.00	10.0	ND

QUALITY CONTROL REPORT

QC Batch No: 100208-1

Analytes	LCS % REC	LCS/LCSD % Limit							
Conventionals									
Solids, Total Suspended (TSS)	109	80-120							



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Project ID: 4088.08.7937.02

Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: SM2540-F, Settleable Solids

QC Batch No: 100108-2

Our Lab I.D.		225768			
Client Sample I.D.		EW-1			
Date Sampled		10/01/2008			
Date Prepared		10/01/2008			
Preparation Method					
Date Analyzed		10/01/2008			
Matrix		Water			
Units		mL/L/hr			
Dilution Factor		1			
Analytes	MDL	PQL	Results		
Conventionals					
Solids, Settleable (SS)	0.100	0.100	ND		

QUALITY CONTROL REPORT

QC Batch No: 100108-2

Analytes	SM Result	SM DUP Result	RPD %	SM RPD % Limit					
Conventionals									
Solids, Settleable (SS)	ND	ND	<1	20					



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Project ID: 4088.08.7937.02

Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: SM4500-CN-E, Cyanide, Total (Colorimetric Method)

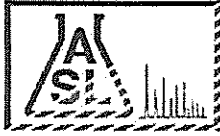
QC Batch No: 100108-2

Our Lab I.D.		225768				
Client Sample I.D.		EW-1				
Date Sampled		10/01/2008				
Date Prepared		10/06/2008				
Preparation Method						
Date Analyzed		10/06/2008				
Matrix		Water				
Units		mg/L				
Dilution Factor		1				
Analytes	MDL	PQL	Results			
Conventionals						
Cyanide	0.0050	0.0500	ND			

QUALITY CONTROL REPORT

QC Batch No: 100108-2

Analytes	LCS % REC	LCS/LCSD % Limit							
Conventionals									
Cyanide	114	80-120							



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Project ID: 4088.08.7937.02
 Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: SM4500-H-B, pH (Electrometric Method)

QC Batch No: 100108-2

Our Lab I.D.			225768			
Client Sample I.D.			EW-1			
Date Sampled			10/01/2008			
Date Prepared			10/01/2008			
Preparation Method						
Date Analyzed			10/01/2008			
Matrix			Water			
Units			pH Units			
Dilution Factor			1			
Analytes	MDL	PQL	Results			
Conventionals						
pH	1.00	1.00	7.36			

QUALITY CONTROL REPORT

QC Batch No: 100108-2

Analytes	LCS % REC	LCS/LCSD % Limit							
Conventionals									
pH	100	95-105							



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Project ID: 4088.08.7937.02
 Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: SM4500-S-2-D, Sulfide (Methylene Blue Method)

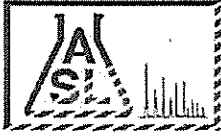
QC Batch No: 100108-2

Our Lab I.D.		225768			
Client Sample I.D.		EW-1			
Date Sampled		10/01/2008			
Date Prepared		10/01/2008			
Preparation Method					
Date Analyzed		10/01/2008			
Matrix		Water			
Units		mg/L			
Dilution Factor		1			
Analytes	MDL	PQL	Results		
Conventionals					
Sulfide, total	0.0100	0.0200	ND		

QUALITY CONTROL REPORT

QC Batch No: 100108-2

Analytes	SM Result	SM DUP Result	RPD %	SM RPD % Limit					
Conventionals									
Sulfide, total	ND	ND	<1	20					



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ANALYTICAL RESULTS

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Project ID: 4088.08.7937.02

Project Name: Melrose Traingle

ASL Job Number	Submitted	Client
39376	10/01/2008	MACOAK

Method: SM5210B, Biochemical Oxygen Demand (BOD)

QC Batch No: 100608-1

Our Lab I.D.	225768						
Client Sample I.D.	EW-1						
Date Sampled	10/01/2008						
Date Prepared	10/01/2008						
Preparation Method							
Date Analyzed	10/06/2008						
Matrix	Water						
Units	mg/L						
Dilution Factor	1						
Analytes	MDL	PQL	Results				
Conventionals							
BOD @ 20C	5.00	5.00	ND				

QUALITY CONTROL REPORT

QC Batch No: 100608-1

Analytes	LCS % REC	LCS/LCSD % Limit							
Conventionals									
BOD @ 20C	110	80-120							

APPENDIX G

WINFLOW® GROUNDWATER MODEL SIMULATIONS

Figure G-1
Regional Groundwater Flow (No Pumping)

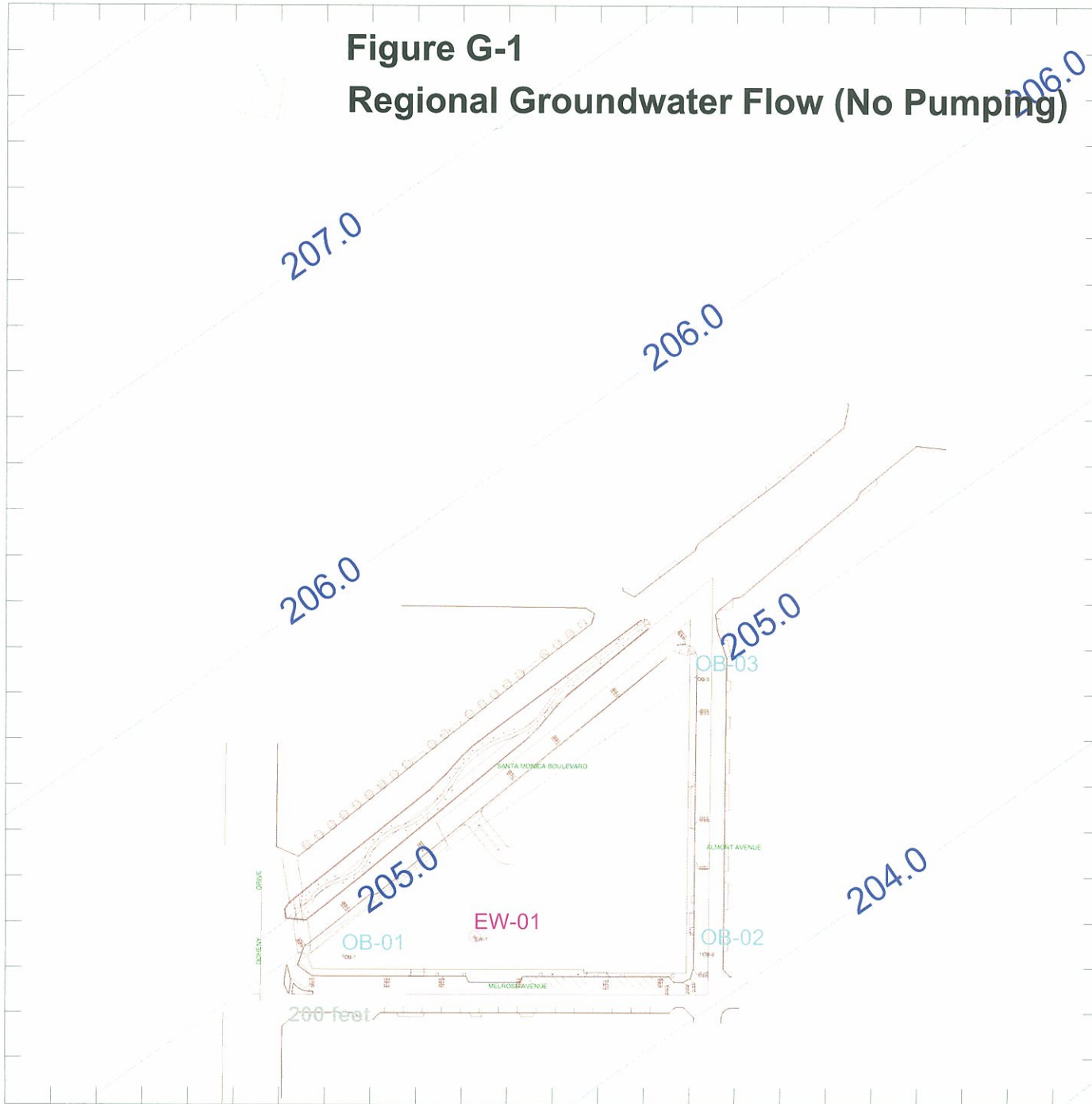


Figure G-2
Simulate Aquifer Pump Test (52 gpm)

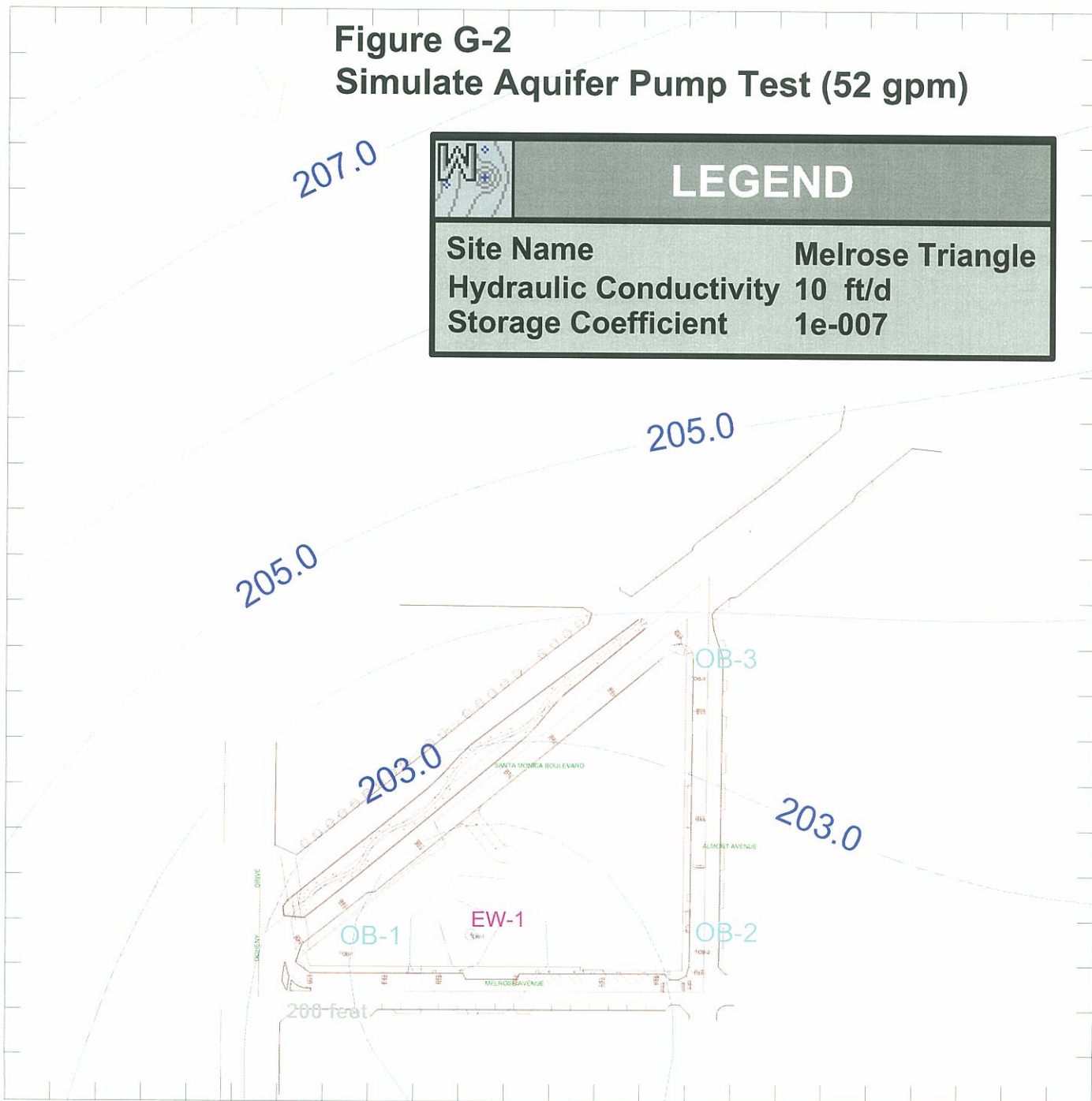
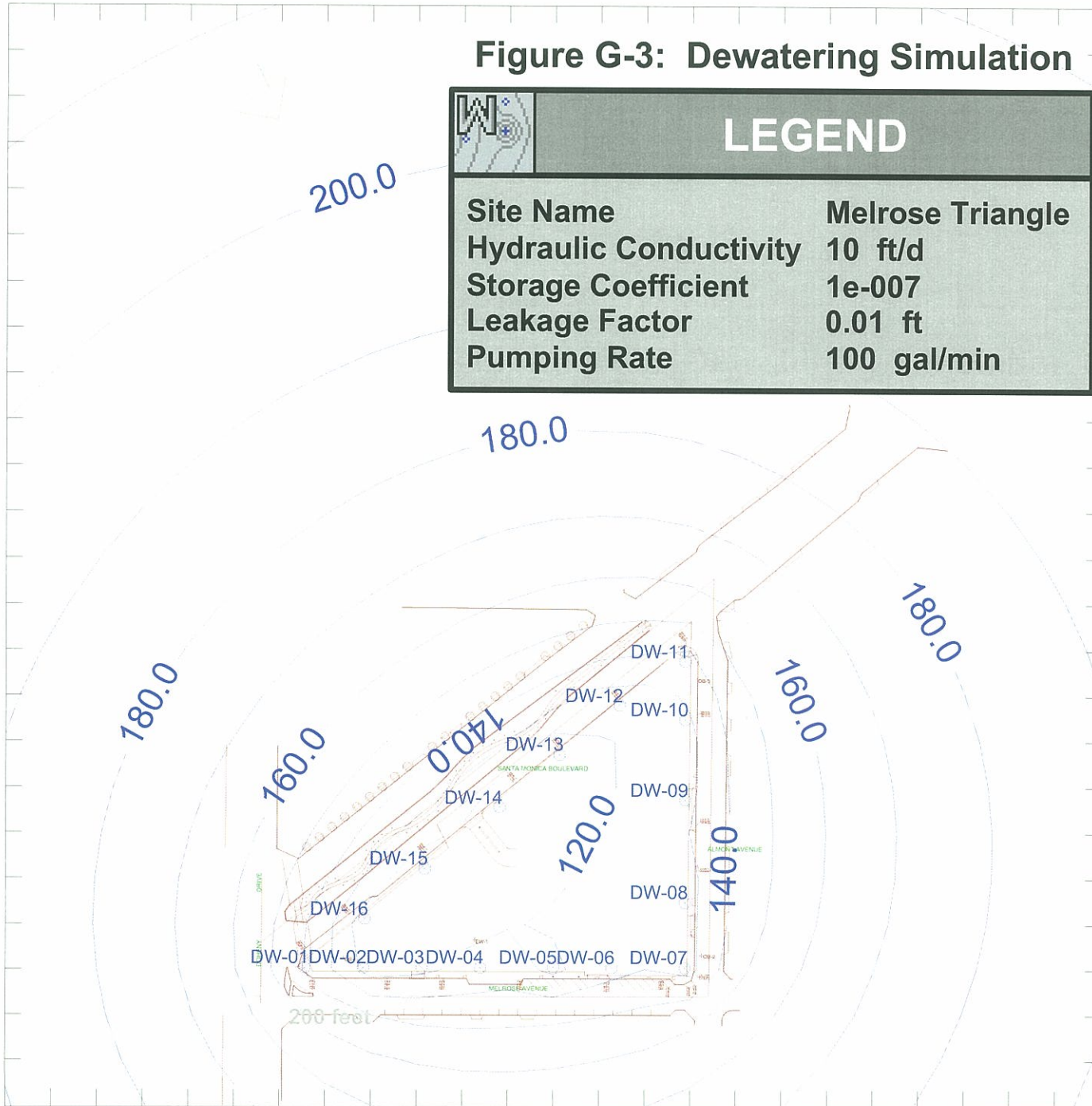


Figure G-3: Dewatering Simulation

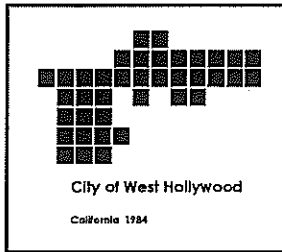
LEGEND	
Site Name	Melrose Triangle
Hydraulic Conductivity	10 ft/d
Storage Coefficient	1e-007
Leakage Factor	0.01 ft
Pumping Rate	100 gal/min



APPENDIX G

WEST HOLLYWOOD WEST RESIDENTS ASSOCIATION COMMENT

LETTER



CITY OF WEST HOLLYWOOD

COMMUNITY DEVELOPMENT DEPARTMENT PLANNING DIVISION

8300 Santa Monica Boulevard
West Hollywood, CA
90069-4313
Tel: (323) 848-6475
Fax: (323) 848-6569

GEOTECHNICAL, GEOLOGY, AND SEISMIC REVIEW SHEET

Site Address: East of Santa Monica Blvd & Melrose Ave **KFMg Project No.:** cWH 05-06E

Lot/Block/Tract: NA

Owner: System, LLC

Project Type: Geotechnical engineering and hydrogeologic evaluation for a new 5-story mixed-use structure with 6-levels of subterranean parking and storage space.

Geotechnical Engineer: MACTEC (Lew, GE 522)

Report Dated: November 28, 2006 (MACTEC Project: 4953-06-2101)

Certified Hydrogeologist: The Source Group (Evensen, C.Hg. 595)

Report Dated: February 6, 2007 (GeoDesign Project: James Hotel-1-01)

STATUS:

Recommend that the following geotechnical and hydrogeologic items be addressed prior to approval by the Planning Department.

GEOTECHNICAL ITEMS TO BE ADDRESSED PRIOR TO APPROVAL:

The geotechnical analyses and recommendations provided in the MACTEC report are generally acceptable and only relatively minor clarifications and further analyses appear to be required based on our understanding of the proposed development. It is noted that the "Melrose Triangle Proposed Construction System Summary Report" prepared by the combined consultants and Turner Construction states that several borings were deferred pending demolition of existing buildings. The Mactec report indicates that 3 borings locations have been deferred. The deferred borings should be performed and the incorporated into a supplemental report. The following items should be addressed prior to geotechnical report approval.

1. p.5 As a matter of statement, based on the provided information the excavation depth could be up to 80 feet deep considering the depth of the finished floor and the excavation needed for installation of the thick mat foundation, waste slab, and possibly some subgrade preparation and improvement.

2. p.6: The Consultant should indicate what kind of hammer (above-ground safety hammer, CME auto-trip hammer, downhole hammer, etc.) was utilized during the drilling programs to obtain the quoted "N-value" blowcounts.
3. p.6 The presented borings were advanced to depths of 100 feet or less. The anticipated depth of dewatering wells (per The Source Group, 2007) is 110 feet. It is recommended that future borings are advanced to depths beyond the well bottom elevation. In addition, as discussed below additional test wells should be installed to further evaluate groundwater chemistry and dewatering conditions.
4. p.7: The Consultant should provide a more detailed analysis and description of liquefaction potential including the considered ground acceleration, identification of liquefiable zones, fines content, and actual Factors of Safety. This analysis is required to determine if seismic increment of lateral pressures of liquefiable soils needs to be considered, as well as liquefaction-induced downdrag on the subterranean walls. As a matter of statement, SP 117 should be adhered to for evaluation of liquefaction potential.
5. p.10 What is the recommended/considered design basis earthquake (DBE) and acceleration?
6. p.10 It is suggested for the Consultants consideration that the estimated 2 - 3 inches of static settlement for structure 70 - 75 feet deep may be excessive.
7. p.10 Indicate how the modulus of subgrade reaction of 50 pci for the mat foundation was determined, i.e., basis, scaling, mat dimension.
8. p.13 The Consultant should clarify if CBC 2001 is the appropriate reference code for this project. It is noted that significant changes to the code have been implemented in the 2007 CBC code.
9. p.14 It is not clear why the EFP groundwater surcharge component below elevation 205 is 37 pcf and not 62.4 pcf. If this value is a result of an analysis considering the change in effective stresses below groundwater, please provide the analysis.
10. p.14 Discussion of seismic lateral pressure increment should be provided. It is noted that the elevation difference across the site may vary by about 11 feet.
11. p.16 If Miradrain is used for waterproofing above elevation 205, is a drainage collection pipe and sump recommended at the bottom of the waterproofing?
12. p.16 The Floor Slab Support section appears to combine recommendations for the basement slab (presumably mat foundation) and the floor slabs for at grade structures. Is the moisture barrier recommended also for the basement slab/mat?
13. p.19 What is the rationale/basis for different shape of lateral loading diagram shown on p.15 and p.19?
14. p.19 Please comment that tiebacks will significantly encroach outside the property limits and into public right-of-way and the potential impacts of such construction.
15. p.20 Clarify the term "lean-mix" concrete. It is suggested that the concrete used for backfilling soldier piles above the dredge line should be readily excavatable to facilitate installation of

lagging during excavation. Lean mix concrete specs often call for compressive strength in excess of 1500 psi, which is typically too strong to be easily excavated.

16. p.20 Please indicate if the soldier pile passive value of 600 psf/ft considers the decrease in effective stresses due to the presence of groundwater. It is noted that the dewatering will presumably lower the groundwater only a few feet below the excavation bottom.
17. p.20 Why is the shaft/skin resistance of soldier piles below the dredge line not improving with depth considering that the resistance of on-site sandy soils will be governed by effective stresses?
18. p.21 Clarify discussion on the use of soldier piles for underpinning of structures. Is it applicable to this project?
19. p.21 It is suggested that the terminology for grouting of tiebacks be consistent with PTI/FHWA terms of "gravity-grouted" and "pressure-grouted" instead of "post-grouted" and "drilled friction" tiebacks used in the text.
20. p.22 It is recommended that the tieback testing recommendations provided in the report could be replaced by a reference to a recognized testing method, e.g., FHWA, PTI. It is suggested that the text in the report may not include all the nuances of a specialized document that might be applicable.

Please discuss if sacrificial testing will be recommended to determine the bond resistance of a permanent tieback.
21. p.25 The use of structural concrete to replace overexcavated areas of the subgrade is questionable, since it creates a localized stiffer response of the mat. It is recommended that the overexcavated material with properties closer to those of the adjacent soil be used instead, e.g., 2-sack concrete slurry.
22. p.25 Discuss where expansive soils are encountered at the site. No test results or visual determination of expansive soils was found in the report.

HYDROGEOLOGIC ITEMS TO BE ADDRESSED PRIOR TO APPROVAL:

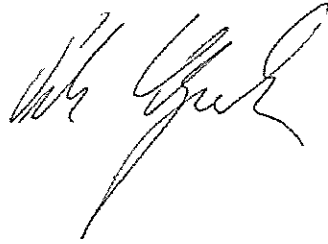
The hydrogeologic and groundwater quality evaluation appear to be preliminary in nature and based on very limited on-site data. No project specific wells have been installed to fully evaluate the proposed dewatering requirements and the water quality within the interval of proposed pumping (110 feet). The Mactec report indicates that 3 geotechnical boring locations have been deferred. It is noted that groundwater monitoring wells could be installed in the geotechnical borings to better characterize the site hydrogeology. The following items should be addressed prior to geotechnical report approval.

1. No information regarding the 2 test wells present at the site are provided. As a minimum, the locations and depths of the existing test wells should be provided. Logs of the wells should also be provided if available.
2. Clearly, a more comprehensive site-specific groundwater evaluation, including a full-scale pump test and further groundwater quality testing, should be implemented to address the extensive dewatering required for the project. It is recommended that the Consultant develop a work plan describing installation a full scale pumping well and required monitoring wells for the pump test

and further water sampling. The work plan should be provided to the City for review and comment prior to initiation.

Reviewed by: Edward H Sabins
Edward H. Sabins, CEG1571
Engineering Geology Reviewer

Date: February 29, 2008



Reviewed by: _____
Peter Skopek, GE 2635
Geotechnical Engineering Reviewer

Date: February 29, 2008



WEST HOLLYWOOD WEST RESIDENTS ASSOCIATION
PO Box 691427
West Hollywood, CA 90069

Phone: 310-659-3379; Fax: 310-659-3380
E-mail: president@whwra.org; website: www.whwra.org

Hand delivered

January 31, 2008

Mr. Jory Phillips
Senior Planner
City of West Hollywood
8300 Santa Monica Boulevard
West Hollywood, CA 90069

RE: Melrose Triangle Project — DEIR

Dear Mr. Phillips:

Thank you for giving West Hollywood West Residents Association the opportunity to comment on the Draft Environmental Impact Report for the Melrose Triangle Project. Attached please find our initial comments and questions regarding the analysis.

Thank you for your time and consideration.

Sincerely,

Lauren Meister
President
West Hollywood West Residents Association

Cc: Planning Commission
Susan Healy-Keene
John Keho

MELROSE TRIANGLE DEIR — PRELIMINARY COMMENTS

Traffic and Circulation

We challenge the Traffic report. It's counter-intuitive that there would not be an impact on the streets where the project is located (Santa Monica Blvd. and Doheny).

There is a dramatic discrepancy between the traffic report in the approved EIR for Greenwich Place and the traffic report in the DEIR for the Melrose Triangle.

Intersection Ratings - Existing Conditions:

	Melrose Triangle DEIR Peak hrs. am & pm	Greenwich Place EIR Peak hrs. am & pm
La Cienega and Santa Monica Blvd. ("SMB")	E & B	F & E
Doheny and SMB	C & B	D & E
Doheny and Beverly Blvd.	C & C	D & E
Robertson and Melrose	A & A	C & E
San Vicente and Beverly	A & B	C & C

If the data on the existing conditions are wrong, how can we trust the projected numbers? When was the study conducted?

We'd like an in-depth analysis of major streets and alleys within a third of a mile of the project, including those in Beverly Hills and LA, based on the numbers presented in the traffic study in the certified DEIR submitted for the Greenwich Place project.

We'd like the study to include traffic on the alley parallel to (just east of) Doheny (from Melrose to Rosewood) and the alley parallel to (just south of) Melrose (from Robertson to Almont). Both of these alleys are currently being used as roadways. Please provide detail about existing use and possible mitigations.

The report does not address local cut-through traffic/avoidance traffic during routine use, during peak-hours or at all. Please provide detail about existing use and possible mitigations.

In the Melrose Triangle DEIR charts, sometimes they use V/C figures and sometimes they use Delay. This is very confusing and we can only assume they're using the number that is the more optimistic/favorable of the two.

Traffic and Circulation (continued)

In the Melrose Triangle DEIR charts, gross averages are used versus peak-hour level of service in the Greenwich Place EIR. Gross averages do not reveal the peak-hour level of service, and we know from the Greenwich Place EIR, which was done in October 2006, that peak-hours for every intersection performance projection is rated "F" for 2008.

We would like data that show us what happens after "F."

Gross averages can mask peak impacts. We would specifically request that peak-hour data also be provided. We would like to see charts by peak-hour level of service for am and pm, in addition to gross averages.

Is there a similar type of storage business in the Los Angeles area from which we can draw data regarding underground specialty storage? Specialty storage seems to be very different from public storage. Traffic patterns, service visits, types of vehicles and operational impacts can be expected to be very different. Please identify real-world data from comparable businesses operating in a comparable area. What types of vehicles will be used to pick up and deliver to the storage units? U-hauls? Trucks of what size?

Regarding the Almont cul-de-sac, the report refers to "traffic calming measures" that can be modified." What does that mean? What do they mean by "participating in the maintenance of the cul-de-sac so that it remains viable"?

What happens to the existing valet parkers who will be displaced during construction? Where will they park all of the cars in the evening?

If the traffic analysis is incorrect and understated, then how can the noise analysis and other related sections (e.g., air pollution) be correct?

Noise

The report talks about Elevado in Beverly Hills and doesn't address Almont or other closer streets in West Hollywood at all. Almont is closest to a major ingress/egress area of the project. Why wasn't that street and surrounding area studied?

If the noise from the Robertson clubs can reach Rangely, why aren't significant effects anticipated from the Melrose Triangle?

What noise levels are expected from the mechanical equipment on the roof?

Will any of the roof be occupied space (like a roof top terrace) and the possible source for noise?

WHWRA Comments
January 31, 2008

Geology and Hydrology

We question the comment that there will be "no subsidence." We've seen it at other nearby areas, such as the Sherbourne/Ashcroft cul-de-sac (i.e., a result of the San Vicente Boulevard storm drain construction).

What is the specific fallback position if they encounter too much water to complete the project as designed? What happens if and when this building becomes so damp as to pose a public health concern?

Excavation is estimated to take 15,000 trips by truck – that will take 400 trips per week, 80 trips per day – for 9 months. What are the truck routes? What are the impacts of the trucks on traffic? What are the impacts of the trucks on the physical roadways?

Will they be using diesel trucks? If so, what are the impacts of the diesel from the 15,000 trips on our air quality? Was there any consideration of using non-diesel trucks and/or zero-pollution vehicles and if so, how would their impact compare to diesel trucks?

Since this is a highly seismic area, has there been any study as to how an earthquake might affect the area once water removal has begun or, once dry, after it has been rehydrated?

We are challenging the comment that the groundwater flow goes Southwest. Based on our own observations, it goes Southeast.

Sub-surface hydrology description and setting is inadequate and does not describe or document the major underground water system that underlies the site or the surrounding area; i.e., amount of water, where it comes from, the flow and where it goes.

The geological and hydrological setting description is incomplete and inadequate. In one section (Geology and Soils) the DEIR claims groundwater was encountered between 21-28 ft below grade. It goes on to cite a previous investigation that found it "as shallow as 12-ft bgs." It then reports a "historic high groundwater level at the site" of approximately 10-ft bgs.

There is no analysis about this wide range or discrepancy nor assessment of the potential for understating or fluctuating prevailing groundwater levels. Another section of the DEIR (Hydrology and Water Quality) relies upon groundwater located approximately 30 ft below grade for its assessment. This is inconsistent with estimates contained within the Geology and Soils section.

Geology and Hydrology (continued)

Further, we believe this reliance is overly optimistic and is contradicted by the report's own citation of the historic high groundwater level of 10 ft. (Our experience is that groundwater levels in the area are much higher and we will address this point later in our comments.)

The DEIR indicates the site slopes in grade 13 ft from west to east and from north to south. Simple math would imply that in areas the natural grade of the site would appear to be below reported historic groundwater levels. The evaluation of this potential is incomplete, inadequate, and relies upon overly optimistic selection of data. Does the groundwater simply dip 13 feet? We request more complete analysis and assessment of environmental impacts. Further, the proposed project is slated to excavate 6 stories below grade. If the reported historic high groundwater level is accurate, most if not all of these stories may be submerged within the groundwater table. If our experience that the groundwater levels are higher is correct, even more of the building will be submerged.

The DEIR uses the metaphor of a bathtub or a boat when discussing construction techniques within such conditions. The DEIR relies upon 100% perfection and certainty about many complicated and sometimes untested building techniques and systems. This is unrealistic. What happens should there be a reasonable 3-5% error rate as is statistically probable and humanly commonplace? What happens if the error rate should prove to be greater? What happens when, as eventually happens even to boats and bathtubs, it springs a leak? To continue the DEIR's metaphors, what happens should the boat pull up its anchors due to inverse pressure expulsion or through other means including ordinary seismic activity?

We know empirically that this site overlays a major underground water system that had sufficient reliable flow to supply the Beverly Hills Water Department water wells on La Cienega for close to a century. There are extensive historic records but the DEIR is silent about it. Please describe this system including source watershed, source flows, routes and dimensions of major aquifers and rivers, flow rates, directional flows, and pressures. This is not adequately described and the impacts of its interference have not been properly analyzed in the DEIR.

We know that Doheny was previously a streambed carrying surface and subsurface flows out of the Santa Monica Mountains and that large surface and subsurface water flows have been observed very recently. High surface flows have caused repeated flooding along sections of Melrose; a nearby portion of the neighborhood is designated by FEMA as a flood plain. While the project site is outside this designated flood plain, it is within the watershed and contributes to runoff that drains into the area that is inside the flood plain.

Geology and Hydrology (continued)

We challenge the DEIR conclusion that there will be no increase in surface runoff. The existing structures contain many varieties of surfaces that hold, diffuse and redirect runoff. The proposed project is more monolithic and would appear to have more impervious surfaces.

We believe there needs to be a complete evaluation of surface water flows, particularly impacts upon gutters and storm channels. Will the project have any impact upon areas downstream including the area already designated as a flood plain? Will increase surface run-off exacerbate surface flows? Due to the topography and grade, area gutters and storm drains are known to overflow during heavy rains and rainy seasons. Is there capacity for extra runoff? How much capacity is there and how much will this project contribute? How much will the project pay to offset this contribution?

Local experience with the high groundwater table is extensive and spans periods of drought and deluge. It has, in fact, spawned epic tales locally. There have been numerous reports of special problems in the area owing to the high groundwater table including subsidence, collapse, flooding, flotation, buoyancy, mold, and the discovery and inadvertent dispersal of hazardous and/or toxic substances including but not limited to oil, tar, explosive fumes, gasoline and oil production residue. The DEIR only makes a glancing mention of one such report and fails to document or review most of them.

These conditions and environmental impacts have not been adequately assessed, described, quantified, evaluated and subsequent mitigation measures discussed in the DEIR. We request this be rectified.

We know high water table conditions have interfered with construction near Doheny above Sunset causing catastrophic structural collapse and at numerous individual locations covering a large area around the project site below Melrose within the same watershed. We know that some local homeowners and developers attempting to build pools encountered water pressure resistance at shallow depths and the pools could not be constructed in ground. County Flood Control had to import special trenching techniques from Southeast Asia to accommodate such conditions for the installation of major flood control pipelines throughout the area. And we personally observed the trenches fill with water to stasis one foot from the surface despite their technology. The DEIR is overly optimistic about the data it has reported and overlooks considerable data it has left out.

We have empirical local experience that groundwater table levels permanently rose immediately to the north following the construction of the Sofitel Hotel on Beverly Place. We know the Hotel must now pump (dewater) 24 hours a day. It would appear the hydrogeology near the Sofitel is likely part of the same system and features as the project site. Yet there is no review, discussion, or analysis.

Geology and Hydrology (continued)

The DEIR lacks any data or modeling to assess similar impacts related to interference with this major underground water system. What happens should the proposed project act like a dam or a huge impenetrable obstacle across this major water system? Will the neighborhood to the north saturate and flood? How much can we expect the groundwater to rise? What happens should the neighborhood to the south, where many mature trees draw from the existing water table, go fallow? What is the projected new route of this water system when it is interrupted with this project? What impact will there be to surrounding properties, streets and major public and private assets? What protection is needed to warrant surety, completion, and indemnification for potential damages? And how much variability is caused by actual accumulated seasonal rainfall?

What are the long-term effects of the underground conditions on liquefaction and on the water table?

When was the test on hydrology conducted? Was it during a run of drought years?

What is the proposed disposal for the discharged groundwater during construction? Why is water in such a parched area not being reclaimed? If long-term dewatering becomes permanently necessary, special mitigation for reclamation and reuse should be formulated and required. Again two sections of the DEIR contradict themselves. The result appears overly optimistic. The Geology and Soils section acknowledges the possibility that permanent operational dewatering is a potential solution to water intrusion. The Hydrology and Water Quality section assumes groundwater withdrawal would not be required during operation and therefore does not provide analysis of this possibility. We would argue this is not just possible but likely. So different scenarios that assess the impacts of ongoing routine withdrawal and disposal of various quantities of groundwater should be fully evaluated.

Hazards and Hazardous Materials

Please explain the disposition of the gas tank, oil wells and any remediation that occurred. Removal of the underground gas tank was so long ago, was it certified to current standards? What about previously capped oil wells?

If no remediation has occurred, what impacts are there on the interface with the underground water system?

We request the DEIR be more forthcoming and complete in the presentation of data relating to THF found in onsite wells. What is the source? How far has it spread? Has it spread beyond the site into the surrounding neighborhood? Does it need a remediation plan and program?

Hazards and Hazardous Materials (continued)

Due to the nature of the hazardous materials known and unknown in and around the project site, we request that a public notification plan be developed as mitigation. Such plan should feature designated onsite safety personnel and an audible alarm and physical, electronic and phone notification system. In addition to notifying responsible agencies, this plan would specify the protocol and responsibility for direct alert of nearby residents, businesses, visitors and the public should contaminants be found during construction that would pose a hazard whether in soil, air- or waterborne.

Population and Housing

The projected population growth from 2005 to 2010 conflicts with the growth projected by the actual number of approved or pending residential units (which is substantially larger). The first set of projections dramatically understates actual population and affects other sections including cumulative impacts, growth-inducing impact, traffic and services, among others. Real data and projections based on real data should be utilized. What is the actual population of West Hollywood now?

Services

The report shows a net addition of 829 units (approved, pending or in the pipeline) that would be completed by 2010. Table 4.10.A ("Demographic projections") shows a net change of 200 from 2005 to 2010. We've already exceeded SCAG's growth projections – with actual growth between 2005 and 2010 being quadruple the SCAG projections.

If the environmental impact analysis is based on SCAG projections, which are clearly understated, then all of the reports on public services and utilities, such as the demand for water, electricity, gas, Sheriff and Fire department, cannot be correct.

Due to current water supply conditions, cities are going to be required to cut back on water use and demand, yet development is moving along as if there are unlimited sources for water. Any new development should be evaluated in the current environment of diminishing water. Why should existing uses be forced to conserve so there can be new development? Shouldn't existing water use and demand have priority over new development that will by definition create new demand?

The letter from the Sheriffs' department (in the Appendix) referred to a previous letter that was submitted regarding the project, but that previous letter is not in the Appendix.

*WHWRA Comments
January 31, 2008*

Land Use and Planning / Long-Term Implications of the Project / Cumulative Impacts

We will be submitting comments about these sections separately.

Historic/Cultural Resources

We would like more information on the architectural value and history of the "streamline" building. Has it been assessed by local, state and/or federal agencies?

Aesthetics

Will the building be over-lit on the Melrose or Almont side?

Will there be light pollution from the building and the apartments?

What is the signage size on the Melrose and Almont sides of the building?

What is the proposed street lighting going to be?

Building Design

Is the Doheny - Melrose - Santa Monica corner going to be effectively a "dead" commercial corner space, given all the exposure to traffic noise and lack of pedestrian traffic?

Is the Melrose - Almont corner going to be effectively another "dead" commercial corner space stranded between all the vehicular traffic entering and leaving the building?

Miscellaneous

List of preparers is inadequate. The public needs to have a list that identifies consultants, including names and companies (Volume 1).

The Appendix is useless as provided. It needs to be indexed and coordinated with the text in Volume 1 as it is impossible to uncover necessary information.